The Salamander Resort and Spa Middleburg, Virginia

Final Report

Paul Roberts Construction Management Advisor: Jim Faust April 2010

The Adamander Resort & Apa

General Building Data

Location: Middleburg, Virginia Occupancy: Mixed use, Hotel, Spa Size: 230,000 sf Height: 4 plus mechanical penthouse Construction Cost: \$93 million Construction Dates: Spring 2008 - Spring 2011



- Basement 5" slab on grade with typical 18x18 and 24x24 concrete columns

- Converts from concrete to steel columns on 1st floor main building
- Guest house 9" or 10" 3500psi post tensioned concrete slab
- Lightgage steel roof trusses at 48" on center
- Spread footings minimum 36" below grade

PAUL ROBERTS CONSTRUCTION MANAGEMENT

Project Team

General Contractor: Turner Construction Company Owner: Salamander Hospitality Architect: Architecture Inc.

Design Architect: Winberly Allison Tong and Goo Structural Engineer: Rathgeber/Goss Associates MEP Engineer: RG Vanderweil Engineers

Mechanical

- 1950 gpm cooling tower on main roof serves chillers 1-3
- 15 AHU's. (9) Variable Frequency Drive. (6) Constant Volume
- (6) additional heat recovery AHU's

Lighting / Electrical

- 3200A 480/277V 3 phase, 4W main switchboard

- Uninterrupted Power Supply (UPS) for 4th floor lodge
- Indoor emergency diesel generator, 650kW 480/277V
- Secondary 120/208V 3 phase

 Dimmable neon 2400K "incandescent" tubing used in spa coves



http://www.engr.psu.edu/ae/thesis/portfolios/2010/pcr115/index.html

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Acknowledgements

Turner Construction

- Mark Miller, Project Manager
- Andy Yohe, MEP Manager

FTI Consulting

• Nick Giambra, Senior Consultant

Penn State Architectural Engineering Faculty

- Jim Faust, Spring Advisor
- Chris Magent, Fall Advisor

Executive Summary

The purpose of this senior thesis is to study The Salamander Resort and Spa, which is located in Middleburg, Va. This report contains a project overview and three analyses focusing on schedule deceleration, guest lodge lighting redesign, and water management. The analyses are focused on reducing the upfront and running costs of the resort through the use of lower energy use and alternate scheduling.

The first analysis deals with the voluntary schedule deceleration per owner's request. The initial design and schedule called for completion in March 2011, but was delayed 12 months to March 2012. In the revised schedule, most activities were not delayed, rather their durations were extended over a longer period of time. The main exception to this schedule was the interior work. From January 2009 to November 2009, all interior work in the lodge was stopped. I analyzed a halt in construction activities for a period of ten months. This will alleviate the general conditions costs for that time period while still allowing the project to finish by March 2012. The general conditions savings totaled \$252,345. The main component of the savings came from the project team salaries and temporary power, lighting, and heating.

The second analysis deals with the redesign of the guest lodge lighting system. A large amount of energy is wasted every year when occupants leave lights on when they are not in the room. The resort has 168 rooms and this leads to a significant energy waste. I analyzed a system that will replace all halogen lamps with LED's and install a control system that will turn off the guest room's lights when no one is present. The total energy cost per year with the LED's is \$5,151 versus \$60,584 with halogens. Taking initial investment, replacement cost, and yearly energy cost into consideration, the payback period for the proposed system is 2.37 years. Approximately \$100,000 will be saved in energy and maintenance costs annually for the following 15 years.

The final analysis investigates the buildings water management, more specifically, the irrigation system. A wide range of plants are used in the surrounding landscaping, many of which are not native to Virginia. Native plants are accustomed to the climate and conditions of the location and are hardier and more likely to survive harsh conditions than that of non-native and exotic plants. By replacing the current pond pump irrigation water source with rain water collection tanks, the system improves sustainability. The additional cost of the proposed system is \$18,350.

Project Overview

Introduction

Building Name: Salamander Resort and Spa Location and Site: Middleburg, Virginia. 340 acres Building Occupant Name: Salamander Hospitality Occupancy: Mixed use. Hotel, spa, equestrian center Size (total square feet): 230,000 ft² Building Cost: \$93 million Dates of Construction: March 2007 – March 2011

Building Enclosure:

Building Facades: There are two major exterior wall facades on the Salamander Resort and Spa. Stone and stone veneer is used on the main entrance building, front and rear. The stone is used on the lower portion of the wall and the stone veneer is used on the middle and upper portion to reduce overall weight. The remainder of the main building and guest wing is stucco.



Roofing:

The roof consists of three different types. Composite slate shingle roofing is used on all slanted roofs. EPDM single ply - fully adhered (TPO) or Modified Bituminous Irma Roofing Systems are used for all flat roofs, usually found in the mechanical spaces.

Client Information

Salamander Hospitality is a company formed in 2005 in part by the current CEO Sheila Johnson. Her goal is to grow the company by acquiring one of a kind properties and managing them to provide an unforgettable experience. Salamander Hospitality specializes in the management of luxury resorts and hotels, like the Salamander Resort and Spa. Their focus on owner and customer satisfaction is what drives this company. Salamander Hospitality also owns and manages the Innisbrook Resort and Golf Club in Florida and the Woodlands Inn in South Carolina. Both of these properties reflect the mission that Salamander Hospitality set out to achieve.

Owner Expectations

Cost: Most of the funding for this project comes directly from Salamander Hospitality. The high-end nature of this project can lead to changes in interior and exterior finishes throughout the project and it is one of the goals of the contractor to minimize the cost impact of these.

Quality: The owner is looking for a very high quality finished product that will serve the needs of her client base. Only the finest fixtures, furniture, and wood details are used. This requires the contractor to provide special attention to the installation process in order to ensure the best looking product.

Schedule: The owner had initially set a Spring 2010 completion date but has since pushed that back to Spring 2011. This has provided the contractor with significant float time, and allows for easier trade coordination.

Safety: It is critical that the contractor provide a safe environment for all the workers on site.



Local Conditions

Figure (2)

Figure (1)

The project is located in the town of Middleburg, Virginia on route 50, about 40 miles west of Washington D.C. and 120 miles north of Richmond (Figure 1). Figure (2) shows the site boundary in blue and the resort location in red.

Preferred Methods of Construction: Much of the residential properties in old town Middleburg are masonry and brick construction. To match this look, Salamander Resort uses a stone façade on the main entrance area.

Construction Recycling: All recycling is collected on site and removed by a third party company to a local recycling plant.

Tipping Fee: In 2008, the tipping fee in Loudon County is \$60/ton. (Loundon County Solid Waste Management Planning District)

Soil Type: The regional soil consists of a blend of deep, well drained, silty soils and clays. During footing excavation, the subsurface water level was not reached.

Project Delivery System

The project delivery method used is a design build. A design build method was chosen because a large portion of the lighting system and custom interior work was not designed at bid time. When Turner took over the project in 2007 it had already undergone three complete redesigns under a different general contractor. The owner initially wanted to fast-track the process to make up for lost ground but this was later altered to fit their needs more accurately.

The Owner, Salamander Hospitality, holds direct contracts with all the design architects, engineers, consultants. The contract between the Owner and Turner Construction is a Cost Plus Fee with a Guaranteed Maximum Price (GMP). Turner Construction holds lump sum contracts with all the subcontractors. The contractor was selected through a competitive process based upon qualification, fee, and a general conditions proposal.

Turner Construction uses a Contractor Controlled Insurance Program (CCIP) which includes workers compensation and general liability. The Owner separately purchased Builders Risk Insurance. There is no Performance Bond on this project.

See the following page for the project delivery system organizational chart.



Staffing Plan

Turner Construction has eight people working on site, one project manager, three superintendents, and four engineers. There are more people on this site than usual because of the complexity of the systems and installation. The Project Engineer and Assistant Engineer are responsible for RFI's and submittals on a daily basis. The Superintendents are responsible for work flow, schedule changes, and subcontractors. Below is the Turner Construction on-site staff.



Building Overview and Systems Summary

Building Systems Summary					
Yes	No	Work Scope	Issues		
	Х	Demolition required?			
Х		Structural steel frame	Mobile crane for erection		
Х		Cast in place concrete	Crane and bucket placement. Wood formwork		
	Х	Precast concrete			
v		Machanical system	Mechanical room located in basement of main		
^			building, northeast corner. Dry sprinkler system		
v	X Electrical system		Main 3200A 480/277 - 3 phase 4W and		
^			secondary120/208V - 3 phase 4W		
Х		Masonry	Stone veneer on main building at entrance		
	Х	Curtain wall			
	Х	Support of excavation			

Excavation:

- All foundations should be a minimum of 36" below grade
- Building spread and strip footings shall bear on undisturbed natural soils or compacted fill with a bearing pressure of 3500 psf.
- Utility lines shall not be placed through of below foundations without structural engineer's approval

Concrete:

- A 3000 psi reinforced concrete was used for 5" interior slab on grade
- The guest house utilized 9" and 10" 3500 psi post tensioned reinforced concrete on metal deck with continuous welded wire fabric.
- Typical 16x28 reinforced concrete columns utilized in guest house.

Structural Steel:

- Rolled shapes and Round HSS Shapes ASTM A992, ASTM A500
- 2" 18 gage Lok-Floor composite metal decking used in the guest house
- 1 ¹/₂" deep, wide rib, 20 gage galvanized roof decking used for both the guest house and main building
- Lightgage steel roof trusses with 8" lightgage purlin at 48" on center

Mechanical System:

- 15 main AHU's, 9 Variable Frequency Drive (VFD) and 6 Constant Volume (CV)
- 6 heat recovery AHU's. 3 located in the main lodge, 2 in the spa, and 1 in the laundry room
- 1950 gpm cooling tower located on the main roof serves chillers 1-3
- Mechanical room located in basement in north east corner

Electrical System:

- From utility, main 3200A 480/277V 3 phase 4W switchboard with secondary 120/208V 3 phase
- Uninterrupted Power Supply (UPS) for 4th floor guest house and 1st floor main building
- Indoor emergency diesel generator (650kW 480/277V 3 phase 4W)
- Custom designed light fixtures and chandeliers

LEED Design Features:

- "Green" slate roofing made from recycled rubber and PVC piping
- Minimize irrigation by using native plants and species
- Maximize opportunity to use building materials made from recycled products
- Use low emitting paints, carpets, and window treatments
- Protection procedures in place to conserve 250 of the 340 acres

Site Plan of Existing Conditions

See Appendix A for Site Plan of Existing Conditions

Site Layout Planning

See Appendix B for Site Layout Planning

The most critical phase of this project is the finishing phase. Due to the large number of custom designed fixtures and materials, transportation and handling should be kept to a minimum to avoid damage. Material storage locations are placed by the guest lodge and restaurant on the northern side of the site, to minimize movement. Turner Construction does not supply any material hoists so the subcontractors should place their hoist in the center section of the guest lodge. The main building and spa is one floor so a hoist is not needed to access those areas.

The on-site trailer and temporary parking is located in the designed parking lot for guests. Dumpsters and recycling collectors are located to the east of the main building, which has relatively flat graded land for easy pick-up. Entrance and exit will on the existing 2-way paved road. Due to the large area and remoteness of the site, no barrier fences are used except for chain link gates at the two entrance and exits.

The planning and coordination for this project benefits greatly from being located on a relatively flat piece of land in the middle of a large open field, free of trees, surrounding buildings, and vehicular/pedestrian traffic. It allows for a lot of freedom when designing a site layout.

Project Cost Evaluation

Actual Costs				
Construction Cost	\$	93,802,046.00		
Construction Cost/SF	\$	409.24		
Total Project Cost Estimate	\$	135,280,000.00		
Total Project Cost Estimate/SF	\$	590.19		

Building Systems Costs (Cost and Cost/SF)					
Building System	Cost Cost/SF			Cost/SF	
Excavation and Fill	\$	277,443	\$	1.21	
Building Concrete	\$	7,191,105	\$	31.37	
Structural Steel and Metal Deck	\$	2,023,292	\$	8.83	
Plumbing	\$	13,766,120	\$	60.06	
Electric	\$	10,674,385	\$	46.57	
Finish Carpentry & Architectural Millwork	\$	4,120,000	\$	17.98	
Windows, Doors, and Glazing	\$	1,285,355	\$	5.61	
Gypsum Drywall Work	\$	5,911,608	\$	25.79	
General Requirements	\$	1,893,275	\$	8.26	

The Total Project Cost Estimate includes land, design/consultant fees, furniture, fixtures, equipment, and development/marketing in addition to construction costs. The Turner Construction Project Manager on site estimated all these additional costs because the owner did not release the official data. As seen in the chart, the most expensive systems in the building are the plumbing and electric.

Three different project cost analysis were performed throughout the course of the fall semester. The most basic estimate was performed using the software D4Cost. The next estimate was done using R.S. Means Square Foot and the final estimate was a detailed structural systems estimate. The three estimates can be seen below.

D4Cost Estimating

See Appendix C for the detailed D4Cost Estimate

The D4Cost estimate was calculated by selecting two similar projects in the database and combining their attributes into one estimate. The closest projects that D4 had in the database were motel/hotels. I chose The Hampton Inn and Suites Hotel because it is a high end hotel located in Chicago. The Inn on Lake Superior is more similar because of the amenities offered but does not come close to the luxury that the Salamander Resort offers.

Projects Used in D4 Cost Estimate					
Use	Project Name	Size (SF)	Floors	Building Cost	
Hotel/Motel	Hampton Inn and Suites Hotel	162,000	12	\$ 13,797,591	
Hotel/Motel	The Inn on Lake Superior	65,345	3	\$ 4,073,012	

Parametric D4Cost Estimate				
Division	Name	Percent	Sq. Cost	Amount
0	Bidding Requirements	7.22	\$ 9.60	\$ 2,200,317
1	General Requirements	3.75	\$ 4.99	\$ 1,144,345
2	Site Work	4.08	\$ 5.42	\$ 1,242,343
3	Concrete	21.00	\$ 27.92	\$ 6,400,073
4	Masonry	6.09	\$ 8.09	\$ 1,855,012
5	Metals	1.56	\$ 2.07	\$ 475,578
6	Wood & Plastics	4.78	\$ 6.36	\$ 1,457,405
7	Thermal & Moisture Protection	1.91	\$ 2.54	\$ 581,897
8	Doors & Windows	6.39	\$ 8.50	\$ 1,947,182
9	Finishes	11.38	\$ 15.13	\$ 3,467,559
10	Specialties	0.35	\$ 0.47	\$ 107,688
11	Equipment	0.09	\$ 0.12	\$ 26,443
12	Furnishings	0.05	\$ 0.06	\$ 13,973
13	Special Construction	0.68	\$ 0.90	\$ 206,514
14	Conveying Systems	2.49	\$ 3.31	\$ 758,084
15	Mechanical	19.61	\$ 26.08	\$ 5,978,605
16	Electrical	8.60	\$ 11.43	\$ 2,620,616
	Total Building Costs	100.00	\$ 132.99	\$ 30,483,633

The D4Cost estimate reported at \$132.99/SF with a total project cost of \$30,483,633. This value is approximately one third of the actual project cost. This significant difference is due in part to the use of the building. The D4 projects are mainly hotel oriented while the Salamander project is a full resort with spa, restaurant, guest rooms, and horse stables/pastures. The Hampton Inn utilizes precast concrete, whereas The Salamander Resort does not. If the three projects had more similar structural, mechanical, and electrical systems the estimate would be closer. The amount of custom interior work, lighting fixtures and woodwork, found on this project also contributes to the difference.

R.S. Means Square Foot Estimate

See Appendix D for the reference pages from R.S. Means 2009

The following R.S. Means square foot estimate is based off M.350: 4-7 Story Hotel with Face Brick and Concrete Back-Up. The costs are calculated using an area of 229,213 square feet and 2,828' perimeter. Basement addition along with height, perimeter, and location adjustments were used. A majority of the structural framing is done with reinforced concrete.

Exterior Moll	S.F. Area	195,000
Exterior vvali	L.F. Area	850
Face Brick with	Steel Frame	\$ 159.60
Concrete Block Back-up	R/Conc. Frame	\$ 157.60

Story Height Adjustment:

12' - 10'3" = 1.75' -\$1.25/ft * (1.75) = -\$2.19/sq. ft.

Perimeter Adjustment:

2828' - 850' = 1978' +\$1.75/100 ft * (1978') = +\$34.62

Basement Addition: +\$32.20/sq ft

Sub-Total Per Square Foot Estimate: 157.60 + 2.19 + 34.62 + 32.20 = \$226.61/sq ft.

> Project Location Adjustment Arlington, Virginia is the closest location listed in RS Means. \$226.61 * 0.93 = \$210.75/sq ft.

Sub-Total Construction Cost \$210.75/sq ft. * (229,213 sq ft.) = \$48,306,640

Common Additives:

(5) 5000 lb. capacity elevators @ \$170,700 each → +\$853,500

(1) Security camera and monitor @ \$1850 and (37) additional cameras @ \$1000 each \rightarrow +\$38,850 (4) 125 lb. washers @ \$32,800 each \rightarrow +\$131,200 (1) 50 lb. washer @ \$12,200 each \rightarrow +\$12,200 (2) Laundry folders @ \$66,500 each \rightarrow +\$133,000 (1) Laundry ironer @ \$35,500 each \rightarrow +\$35,500

Common Additives Total = +\$1,204,250

Total Construction Cost:

48,306,640 + 1,204,250 = 49,510,890

\$216.00/SF

The R.S. Means square foot estimate is about \$45 million less than the actual project cost. Part of this difference can be attributed to using a hotel as the basis for the estimate, as R.S. Means does not have a category for resorts. Another reason for the difference comes in the façade, R.S. Means uses Face Brick with Concrete Clock Back-Up while The Salamander Resort uses a stone veneer and stucco. Other discrepancies were discussed in the analysis of the D4Cost estimate.

The R.S. Means estimate for this project is more accurate than the D4Cost estimate partly because the Means estimate is more tailored to this project, while the D4 Cost is based off other buildings. While neither estimate had a good basis for comparison, the R.S. Means estimate is would be fairly accurate if it included more of the specialty items and finishes found in The Salamander Resort.

Detailed Structural Systems Estimate

See Appendix E for detailed structural systems estimate

Total Structural Costs						
System	SF		\$/SF		Cost	
Concrete	230000	\$	1.96	\$	449,821.00	
Structural Steel	230000	\$	2.93	\$	672,769.00	
Reinforcing	230000	\$	1.04	\$	238,934.00	
Sub-Total	230000	\$	5.92	\$	1,361,524.00	
		Location Factor			0.982	
			Total	\$	1,337,016.57	

Assumptions:

- Location Factor, Arlington = .982
- 2 use plywood was used for forming
- No waste factors were used
- Footings used 6 #6 for reinforcing
- Slab on grade used #4 @ 12" O.C. for reinforcing
- Concrete Beams used 6 #7 for reinforcing
- Concrete Column used 8 #10 for reinforcing
- Elevated slab used #4 @ 24" O.C. for reinforcing
- Concrete CY totals do not exclude volume of rebar

The detailed structural estimate was performed using R.S. Means 2009. Due to the irregularity of my project I was unable to do a simple estimate of a typical bay and extrapolate. The guest lodge is the only area that has a repeatable structural system. In order to simplify the take-off of concrete beams, concrete columns, and steel members, I used a length range method. I grouped all the different sized beams and columns into length ranges. For example, I counted up all 24"x24" concrete beams and categorized them as either 10'-15', 15'-20', 20'-25', etc. I then took the average length, in this case 12.5', 17.5', 22.5', and multiplied it by the quantity and size to get cubic yards of concrete. I used a similar method for the concrete columns, footings, and steel members. As seen in the above assumptions, I used uniform reinforcing for slabs, beams, and columns to simplify to the take-off. The total structural cost for the project is \$1,337,016.57.

The actual cost of building concrete from the GMP estimate by Turner Construction is \$7,191,105. This number is significantly larger than the value that I obtained for structural concrete. The main reason for this difference is that this work was performed by a subcontractor

who also had to excavate and backfill the footings. The actual estimate also includes concrete used for paving, sidewalks, and retaining walls. I also did not take into account the additional material and labor costs of post tensioned concrete in the guest lodge.

General Conditions Estimate

See Appendix F for General Conditions Estimate

Assumptions:

- Location factor, Arlington = .982
- Turner Construction employees are on site for entire duration of project
- Project duration: 5 years = 60 months

General Conditions Summary				
Item Cost % of GC				
Field Personnel	\$ 2,419,402.50	50.8%		
General Expenses \$ 876,418.50 18.4%				
Temporary Utilities	\$ 209,274.81	4.4%		
Insurance \$ 1,260,298.80 26.49				
Total	\$ 4,765,394.61	100.0%		

The General Conditions estimate was performed using R.S. Means 2009. The estimate was broken up into four categories, field personnel, general expenses, temporary utilities, and insurance. The estimate came to \$4,765,394.61 which is 5.1% of the total construction cost. The largest portion of the cost estimate, roughly 50%, is from field personnel because Turner Construction has seven employees on-site.

Detailed Project Schedule

See Appendix G for a detailed Project Schedule

The Salamander Resort and Spa schedule is broken up into the construction of three buildings, the guest lodge, the spa, and the main building.



The three buildings each begin and finish construction at about the same time. The important dates are shown below.

Building	Start	Finish	Duration (days)
Lodge	1/23/2008	12/19/2011	976
Spa	2/19/2008	9/8/2011	928
Main Building	2/27/2008	11/30/2011	980

Construction of the spa and main building are very similar except for the inclusion of more structural steel in the main building. Project Substantial Completion occurs less than a week after the finish of the main building. Closeout takes roughly two months, and the building is handed over to the owner in March 2012. In 2008 the schedule was modified and delayed by a year to accommodate the owner's wishes. This change can be seen very clearly in the delay between the structure of the buildings and the finishes. Between January and November 2009, all interior work was put on hold. After the schedule adjustment, the total duration of the project is exactly five years, March 1, 2007 to March 2, 2012

Foundation

The total duration of the foundation work was approximately 6 months. The foundation consisted of reinforced concrete spread footings excavated down to a minimum of 36 inches below the slab on grade. No formwork was needed because the excavation holes were dug to the correct footing size.

Structural

Superstructure for The Salamander Resort and Spa took about 6 months to complete. It consisted of both concrete and steel framing. The basement and guest wing have concrete

framing throughout and the main building has steel framing on the first floor. This created challenges in the schedule when both concrete and steel were being installed simultaneously.

Finishes

Turner has allocated a large amount of time to the finishes due to the complexities of the project. A majority of the fixtures are custom designed for this project and are more likely to require additional time to install. Like many of the activities, finishes would be able to finish in less time than the allotted if the schedule was optimized for time.

Analysis I: Schedule Deceleration

Introduction

The initial design and schedule for The Salamander Resort and Spa called for completion in March 2011. Per owner's request, the project was delayed 12 months to March 2012. This was done for a variety of reasons that will be discussed later. This intentional deceleration of the schedule creates more work for the contractor. The cost impact is significant, mainly due to the additional year of general conditions. For this analysis, I am proposing a halt in construction activities for a period of ten months. This will alleviate the general conditions costs for that time period while still allowing the project to finish by March 2012.

Original Schedule: Completion date March 2011

The Salamander Resort and Spa schedule involves the construction of three interconnected areas, the guest lodge, the spa, and the main building. The breakdown of areas is seen in the schematic below.



The Main Building and Spa are smaller in square footage than the lodge because they are one floor compared to four. Due to the repetitive nature of the guest lodge, the three building areas all finish in roughly the same amount of time, from exteriors to interiors.

The critical path for The Salamander Resort and Spa followed that of a typical commercial construction project. It includes the following activities; concrete frame, steel frame, core/shell, enclosure, and interiors. A delay to any of these activities would cause a delay in the overall project completion date. The roof dry-in milestone was the most critical point in the schedule as it occurred between the enclosure and the interiors. This was an important point to reach

because it then allowed the extensive interior work to begin. The importance of this milestone was later negated due to the one year delay the owner placed on the entire project.

Revised Schedule: Completion date March 2012 See Appendix G for Revised Schedule

The revised schedule in Appendix G is the schedule created immediately following the decision to extend the project by 12 months. Therefore, it shows a gap in activities similar to that of my proposed schedule in Appendix H. What differs between this revised schedule and the actual work is that the activities preceding the break were not on schedule and ran into the extension period.

The revised schedule has a project completion date of March 2012. The owners of The Salamander Resort and Spa decided it was in their best interest to delay the project by one full year. One of the main reasons for this decision was the current economic climate. They felt that if the resort opened up in March 2011, and the economy had not recovered, they could not rent out all the rooms to capacity. The owner of Salamander Hospitality, Sheila Johnson, is a prominent figure in the Middleburg community, and she felt that this would be bad for her image. The name of her company, Salamander Hospitality, is part of the name of the resort and this would be a direct negative tie to any future properties or communities the company would build.

Deceleration of a construction project is a very rare occurrence and is usually only done in extreme cases. There are numerous negatives aspects to the deceleration of a project schedule. When it comes to timeline and substantial completion dates, owners almost always want them to be earlier. They want to begin collecting rent from tenants or use the building themselves sooner rather than later. When a project is unintentionally delayed, it can cost the owner thousands of dollars a day. Clauses are sometimes built into the contract to make the contractor pay for lost profit if they don't finish on schedule. On this project, because the owner requested the delay, a different set of problems arose.

One of the problems that resulted from the deceleration of this project was the renegotiation of contracts between contractor and subcontractors. The two parties had to agree to when the required work will be completed and more importantly to the subcontractor, when they would be paid. The largest expense of the deceleration to the owner came from cost of the salaries of the Turner Construction project team for the additional 12 months. Temporary lighting, heating, and power are required for all the additional months and can be a large expense during the winter months.

For the revised schedule, most activities were not delayed rather their durations were extended over a longer period of time. The main exception to this schedule was the interior work. From January 2009 to November 2009, all interior work in the lodge was stopped. By the end of December 2008, a portion of the metal framing had been completed, and on January 1, 2009 the interior work was put on hold for 11 months. When it was stopped, some of the interior work had been completed, requiring heating and cooling during the shutdown period.

Proposed Schedule

See Appendix H for Proposed Schedule

I am proposing that instead of the revised schedule, where select activities were lengthened and interior work was put on hold, all activities be stopped for a period of approximately 10 months. Ten months instead of the full twelve months is chosen because certain activities should be completed prior to shutting down the site.

The shutdown of the site will occur between the shell and core completion dates for all the building areas and the start of the interior work. Interior metal framework will be included in the shell and core completion. The project shutdown will be for 43 weeks and occur between December 21, 2009 and October 18, 2010. The important dates of the proposed schedule can be seen below.

Important Dates					
Complete Shell and Core Start Finish Complete Work Work					
Lodge	9/24/2009	10/29/2010	11/16/2011		
Spa	9/1/2009	12/27/2010	11/10/2011		
Main Building	12/18/2009	10/19/2010	11/30/2011		

The main building core and shell is completed last because it has the most complicated façade and requires additional installation time. Due to the extensive interior work, the finish work for all three areas take approximately 12 months to complete.

The interior finishing start and finish dates were modified to allow for the overlap of same trades across building areas. This was done so a subcontractor could work straight through and not have week breaks between the main building work and the spa work. An example of this overlapping is shown below with the Hanging/Taping/Finish of the drywall.

Hang/Tape/Finish Drywall Dates						
	Start Date Finish Da					
Main Building	12/21/2010	3/15/2011				
Spa	2/1/2011	2/21/2011				
Lodge	2/14/2011	6/7/2011				

This analysis was done for all the finish work from Layout to MEP Trimout.

The main difference between my proposed schedule and the revised schedule is that the revised schedule still maintains the full Turner Construction project team on site. By removing the project team, the salaries and the office general conditions are eliminated. The temporary power can be reduced by 90%, while the temporary lighting can be reduced by 80%. As for the temporary heating, 30% is needed for the months December to March to keep the interior temperature at 40 degrees, and 0% is needed for April to November. In a finished space, humidity and temperature are a huge factor. Mold can grow on drywall, carpets, and curtains if the humidity is not correct. I eliminate this problem by placing the break in the schedule before any of these sensitive materials are installed. This was one of the contributing factors to choosing the dates of the 10-month halt.

I consolidated related activities that were broken up across the revised schedule. The Ecostar Slate roof installation was originally scheduled for installation from August 30, 2010 to October 7, 2010 for the spa and from September 13, 2010 to November 5, 2010 for the main building. I moved it to immediately following the roof installation of spa and main building. This allows the roofing subcontractor to finish all his work instead of postponing approximately 30 work days.

Below is a summary of the general conditions saved during the 43 week break. See Appendix I for a detailed breakdown of the general conditions with the proposed schedule.

General Conditions Savings							
Description	Description						
Field Perso	\$ 423,765.00						
General Ex	\$ 24,700.00						
Temporary	Utilities	\$ 47,155.12					
	Sub-Total	\$ 495,620.12					
	Location Factor	0.982					
	Total	\$ 486,698.96					

Offsetting some of the savings are additional security expenditures. Security is needed on a full time basis to protect against theft and vandalism. A single security guard on two 12-hour shifts will be required. The total cost of security for 43 weeks is \$234,354.30. See Appendix J for a detailed breakdown of security. This offsets approximately 48% of the overall general conditions savings.

Recommendation

The following chart summarizes the overall general conditions savings and additional cost.

Overall Savings						
General Condition	\$	486,698.96				
Temporary Secu	\$	234,354.30				
	Total Savings	\$	252,344.66			

The overall savings from my proposal are approximately a quarter of a million dollars. On a project that was heavily value engineered before and during construction, this amount is significant and this timeline should be considered as an alternative to the revised schedule. The 10-month break allows the owner to consider any other value engineering topics without the time restrictions. Turner Construction also benefits from this because it allows them to rearrange manpower and place the current staff on other projects.

Analysis II: Guest Room Lighting Redesign

Introduction

As with most hotels and resorts, a large amount of energy is wasted because occupants leave the lights on when they are out of the room. The Salamander Resort and Spa has 168 guest rooms and the likelihood of everyone turning off unneeded lights is very low. When in the room, occupants also tend to use more light than is necessary, having multiple light fixtures on at once. With the introduction of compact fluorescent and LED lights in recent years, the ability to significantly reduce energy bills has increased. Specialized lighting control systems also increase efficiency and reduce cost.

The resort has 168 guest rooms in seven different layout types, king, ADA king, double queen, ADA double queen, junior suite, executive suite, and presidential suite. The breakdown of the room types is shown below.

	Number of Rooms						
Room Type	Ground Floor	1st Floor	2nd Floor	3rd Floor	Total		
King	22	24	29	27	102		
ADA King	1	1	1	1	4		
Double Queen	12	13	8	8	41		
ADA Double Queen	2	1	1	0	4		
Junior Suite	3	3	3	3	12		
Executive Suite	1	1	1	1	4		
Presidential Suite	0	0	0	1	1		
				Total	168		

For this analysis, the room types are broken into two layouts, king/queen (151 rooms) and suites (17 rooms). The lighting layout of the junior suites is used for all the suites.

Current System

The current lighting system for The Salamander Resort and Spa is highly customized. Halogen lamps were chosen for their increased life expectancy and high output over normal incandescent lamps. The guest rooms are predominantly halogen downlights that utilize Par20 lamps. The ceiling and wall fixtures for the space are not specified but designed wattage is given. I will assume these fixtures use Par16 halogen lamps. The number of lamps needed is determined by the designed wattage. The lamp specifications for the Par20 and Par16 lamps can be seen below:

Par20 Halogen		Par16 Halogen	
Approx. Lumens	570	Approx. Lumens	450
Average Rated Life (hr)	2500	Average Rated Life (hr)	2500
Beam Type	Flood	Beam Type	Flood
Beam Angle	25 deg.	Beam Angle	
Diameter (in)	2.5	Diameter (in)	
Diameter (mm)	63.5	Diameter (mm)	
Filament	CC-8	Filament	
Maximum Overall Length (in)	3.125	Maximum Overall Length (in)	
Maximum Overall Length (mm)	79.3	Maximum Overall Length (mm)	
Nominal Voltage (V)	120	Nominal Voltage (V)	120
Nominal Wattage (W)	50	Nominal Wattage (W)	45
Price	\$5.50	Price	\$7.95

It is important to note the lumens, average rated life, nominal wattage, and price for both lamps. The Par20 lamps produce 570 lumens while the Par16 puts out slightly less, 450 lumens. The average rated life is 2,500 hours which is typical of most halogen lamps. The dimensions of the Par16 lamp will be determined when the fixture is designed. The Par20 costs \$5.50/lamp and the Par16 is \$7.95/lamp. The price is for the lamp alone and does not include installation costs. The wattage for both lamps is around 50 watts. This value is similar to that of a comparable incandescent lamp.

The current fixture schedule is shown below for both the typical king/queen room and junior suite:

Typical Double Queen/King Guestroom (151 Rooms)									
Tupo	Description	Quantity of	P	er Lamp		Quantity of	Volt Amos		
туре	Description	Fixtures	Voltage	Watts	Amps	Lamps	voit-Amps		
AM	Adj. Downlight	2	120	50	0.42	2	100		
AN	Downlight	2	120	50	0.42	2	100		
AP	Shower Rated Downlight	2	120	50	0.42	2	100		
AR	Downlight	2	120	50	0.42	2	100		
GD1	Decorative Wall Fixture	1	120	45	0.38	1	45		
GD2	Decorative Ceiling Fixture	1	120	45	0.38	3	135		
GD3	Decorative Wall Fixture	2	120	45	0.38	2	90		
GD4	Decorative Ceiling Fixture	1	120	45	0.38	2	90		
						Total/Room	760		

There are a total of 8 downlight fixtures with one lamp in each. There are also 5 decorative ceiling and wall fixtures that combine for a total of 8 lamps. GD2 and GD4 have multiple lamps because the designed wattage is greater than the wattage of one lamp. Three lamps are put in the GD2 ceiling fixture and 2 in the GD4 ceiling fixture. The number of volt-amps was calculated for each fixture and then summed for the whole room. The total number of volt-amps for one king/queen room is 760 VA.

Typical Junior Suite (17 Rooms)									
Tuno	Description	Quantity of	Р	er Lamp		Quantity of	Volt Amos		
туре	Description	Fixtures	Voltage	Watts	Amps	Lamps	voit-Amps		
AM	Adj. Downlight	2	120	50	0.42	2	100		
AN	Downlight	2	120	50	0.42	2	100		
AP	Shower Rated Downlight	2	120	50	0.42	2	100		
AR	Downlight	2	120	50	0.42	2	100		
GD1	Decorative Wall Fixture	1	120	45	0.38	1	45		
GD2	Decorative Ceiling Fixture	1	120	45	0.38	3	135		
GD3	Decorative Wall Fixture	0	120	45	0.38	0	0		
GD4	Decorative Ceiling Fixture	2	120	45	0.38	4	180		
						Total/Room	760		

The junior suite fixture schedule is similar to the king/queen rooms. The only difference is the number of decorative wall and ceiling fixtures. Despite this difference, the number of lamps is unchanged. When the volt-amps are summed across all lamps in the junior suite the room total is 760 VA.

For each room type the volt-amps are the same, 760 VA. This was applied to all the rooms in the lodge and converted to kW to get the total energy use.

kW for 151 Rooms					
114.76					
kW for 17 Suites					
12.92					

For all the king/queen rooms in the lodge the total energy use is 114.76kW, while the suites consume 12.92 kW. I will compare costs below.

Proposed System

The proposed system will replace all halogen lamps with LED's and install a control system that will turn off the guest room's lights when no one is present. The two manufacturers used are EarthLED for the lamps and Messerschmitt for the control.

LED lamps are chosen over compact fluorescent because CFL's contain mercury and must be treated as hazardous waste upon disposal. Modern LED lamps are able to replicate the light emitted by incandescent and halogen bulbs at a significantly lower wattage. This increased efficiency leads to direct energy cost savings. Other benefits of LED lamps include instant full brightness upon startup, no output of ultraviolet light, reduced maintenance cost due to long life, and less heat production. The main disadvantage of LED lamps is the relatively high bulb cost. This can deter many of potential buyers who are unfamiliar with the substantial energy savings.

Two different types of LED bulbs will be installed, both of which are manufactured by EarthLED. The lamps are direct replacement retrofits that do not require any addition equipment for installation. The lamp to be installed in the downlights is "EarthLED Lumiselect Par20/R20 Dimmable LED". The "EarthLED Lumiselect Par16/R16 Dimmable LED" will be installed in the decorative wall and ceiling fixtures. Lamp specifications can be seen below.

EarthLED LumiSelect PAR20/R20 Dimma	ble LED	EarthLED LumiSelect Par16/R16 Dimmable LED			
Approx. Lumens	450	Approx. Lumens	300		
Average Rated Life (hr)	50,000	Average Rated Life (hr)	50,000		
Beam Type	Flood	Beam Type			
Beam Angle	90	Beam Angle	90		
Diameter (in)	2.91	Diameter (in)	2.36		
Diameter (mm)	74	Diameter (mm)	60		
Filament		Filament			
Maximum Overall Length (in)	4.01	Maximum Overall Length (in)	4.25		
Maximum Overall Length (mm)	102	Maximum Overall Length (mm)	108		
Nominal Voltage (V)	120	Nominal Voltage (V)	120		
Nominal Wattage (W)	9	Nominal Wattage (W)	6		
Comparable Wattage (Incandescent)	50-60	Comparable Wattage (Incandescent)	50		
Price	\$90	Price	\$70		

The three most important differences between the halogen and LED lamps are average rated life, nominal wattage, and price. The average LED lamp is rated for 50,000 hours, 20 times longer than the specified halogen lamp while using less than a fifth of the energy. The lumen output is slightly less than the halogen counterpart but EarthLED claims the usable light output is similar

to that of a 50 watt incandescent lamp. The LED's cost significantly more than the halogen lamps and the payback period will be analyzed later in this section. A factor that determines which lamp is chosen is the ability of dimming, as the original halogen lamps are dimmable.

One of the largest energy drains in the hospitality industry is hotel guests leaving lights and electronics on in the room when they are not present. In most hotel settings, guests are absent from their room for extended periods of time during the day. To combat this problem, this analysis is proposing the use of a stand-alone guest occupancy key card system. Upon entering the room, the guest places the hotel room key in a card reader. When the card is in the reader, electricity flows to all connected lights and electronics. When the guest leaves, they take the key card and all connected lights are turned off to save energy. This is similar to many hotel lighting control systems found in Asia. For this analysis, it is assumed that the guests are absent from their rooms between the hours of 10 am and 4 pm.

Cost Analysis – Electrical Breadth Analysis

Energy Cost

On initial inspection, it might seem that the initial cost of the LED lamps far outweighs the long term energy cost savings, but one has to remember that it is not only the energy savings from the lamp but also the lighting control system that keeps them on for about half the time of the halogens. The chart below shows the energy cost per year broken down by lamp type. The quantity of lamps, 1344, is the total number lamps of each type in the guest lodge at any one point. Calculating the energy use of the halogen lamps per kWh assumes that the lights will not be turned off during the unoccupied or occupied time frame. The proposed system with LED lamps will shut off during the unoccupied time frame, 10 am to 4 pm. The cost per kWh in Middleburg, Va is \$0.10.

Current Energy Use (Halogen)										
Lamp	Quantity of Lamps	Watts /Lamp	Total Watts	Total kW	Unoccupied 10AM - 4PM	Occupied 7 hrs.	kWh /day	\$ /kWh	\$ /day	\$/year
50PAR20H/FL25	1344	50	67200	67.2	403.2	470.4	873.6	0.10	87.36	31886.40
45PAR16/FL	1344	45	60480	60.5	362.9	423.4	786.2	0.10	78.62	28697.76

Proposed Energy Use (LED)										
Lamp	Quantity of Lamps	Watts /Lamp	Total Watts	Total kW	Unoccupied 10AM - 4PM	Occupied 7 hrs.	kWh /day	\$ /kWh	\$ /day	\$/year
EarthLED LumiSelect PAR20/R20 Dimmable LED	1344	9	12096	12.1	0.0	84.7	84.7	0.10	8.47	3090.53
EarthLED LumiSelect Par16/R16 Dimmable LED	1344	6	8064	8.1	0.0	56.4	56.4	0.10	5.64	2060.35

Existing kWh/day	Existing \$/year			
1659.84	\$	60,584.16		

Proposed kWh/day	Prop	oosed \$/year
141.12	\$	5,150.88

The annual savings on energy alone by installing the lighting control system and alternate lamps is over 55 thousand dollars. Before the recommendation can be made to implement this system, one has to look to factor in the bulb and maintenance costs.

Halogen Replacement Cost

With a 2,500 hour lamp life and 13 hours of use per day, one halogen lamp will last approximately 6.5 months. This comes out to 1.9 halogen lamps per year, per fixture. Maintenance costs come into play when replacing the bulbs. Assume 10 lamps can be replaced every hour and at \$20/hr, the cost per lamp is an additional \$2. The table below shows the cost per year to replace halogen lamps.

Halogen Replacement Cost					
Room Type	Lamp Туре	Lifetime Lamp Hours	Hrs. in use/year (Fixture)	Lamps/year	
Typ. King/Queen	50PAR20H/FL25	2500	4745	1.90	
Typ. King/Queen	45PAR16/FL	2500	4745	1.90	
Typ. Suite	50PAR20H/FL25	2500	4745	1.90	
Typ. Suite	45PAR16/FL	2500	4745	1.90	

Halogen Replacement Cost						
Room Type	\$/Lamp + Install*	\$/Year/Lamp		Lamps/Room	Rooms	\$/Year Total
Typ. King/Queen	\$ 7.50	\$	14.24	8	151	\$ 17,195.88
Typ. King/Queen	\$ 9.95	\$	18.89	8	151	\$ 22,813.20
Typ. Suite	\$ 7.50	\$	14.24	8	17	\$ 1,935.96
Typ. Suite	\$ 9.95	\$	18.89	8	17	\$ 2,568.37

*Install Cost: 10 per hour at \$20/hr = \$2/lamp

Total Cost/year \$ 44,513.41

The total cost per year to replace the halogen bulbs comes out to slightly over 44 thousand dollars. This value includes the initial investment of purchased lamps.

Initial Investment

The next thing to consider is the initial investment of the proposed system. With a 50,000 hour lamp life and 7 hours of use per day, one LED lamp will last approximately 17 years. The following chart shows the cost to buy and install the LED lamps in all the guest rooms. Unlike the halogen replacement chart, this is a one-time initial cost calculation as the next time the lamps will need to be replaced is after 17 years.

Lamp Type	Lamps /Room	Rooms	Lamps	\$/Lamp + Install	Cost (\$)
EarthLED LumiSelect PAR20/R20 Dimmable LED	8	168	1344	\$ 92.00	\$ 123,648.00
EarthLED LumiSelect Par16/R16 Dimmable LED	8	168	1344	\$ 72.00	\$ 96,768.00

The key card system plus installation will be \$100 per room, which totals to \$16,800 for all the guest rooms.

Total Initial				
Proposed Cost				
\$	237,216.00			

Operating Cost

The total proposed lighting system including lamps, install, and control system cost comes out to a little over a quarter of a million dollars. The proposed initial and energy cost is more than the halogen annual replacement and energy cost. The final piece of information to look at is the payback period for the existing versus the proposed.

		Operating Costs/year		
	Initial Investment	Replacement Cost^	Energy cost	
Existing	0*	\$44,513.41	\$60,584.16	
Proposed	\$237,216.00	0	\$5,150.88	

*included in annual replacement cost ^per year for 17 years

To calculate the payback period to recoup the initial investment of the new system one must add the replacement and energy cost of the existing system, subtract from that the energy cost of the proposed system and then divide that number into the proposed initial investment. The final payback period comes out to 2.37 years.

Recommendation

The initial investment of the proposed system is significantly higher than the current system, but with \$55,000 annual savings in energy and \$44,000 less per year in replacement costs, the system has a payback period of 2.37 years. Following the payback period, the annual savings is approximately \$100,000 for the following 15 years, or until the LED lamps burn out. Replacing all the LED lamps costs \$220,416.00, this is more than made up for in the 15 previous years of energy and replacement savings.

Analysis III: Water Management

Native Plants

The Salamander Resort and Spa employs an immense number of different trees, shrubs, vines, perennials and annuals. This is done to create a diverse and unique look around the building. One objective of this analysis is to create a similar look to the originally designed plants but replace them with native plant species. Using native plants has many benefits, the largest being that they are able to survive on natural rainfall and don't need an irrigation system to prosper. Native species are plants that are present in the region in which they have evolved. They are accustomed to the climate, rainfall, soil, frost, and interactions with other species. Native plants have developed a special set of characteristics that allow them to prosper in the current region without the use of fertilizers or pesticides. They also have the ability to match the quality and aesthetics of invasive and exotic plants while surpassing them in durability, resistance to insects, and disease.

One of the most important characteristics of native species in relation to this analysis is the ability to survive and prosper on the natural rainfall. I have chosen to just focus on replacing shrubs because they draw the most water in the areas where irrigation is placed. The vast majority of trees lie outside of the reach of the irrigation system because they are more tolerant of drought conditions. The perennials and annuals placed around the site provide the Resort with a very unique look that is hard to replicate. They also require the least amount of water on a per plant basis. One issue with performing a water savings analysis is that it is nearly impossible to quantify the actual savings.

According to the Virginia Department of Conservation and Recreation there are close to sixty different species of native shrubs in the state of Virginia. Below is a list of the proposed shrubs along with the native alternative.
Quantity	Proposed	Native Replacement			
277	Fothergilla	Buttonbush			
71	Cherry Laurel	Virginia Sweetspire			
74	Otto Luyken Cherry Laurel	Henry Garnet's Sweetspire			
64	San Jose Holly	Red Chokeberry			
19	Korean Spice Viburnum	Arrowwood Viburnum			
5	Shasta Doublefile Viburnum	Possumbaw Viburnum			
91	Nandina	Inkberry			
27	Vernal Witchhazel	Common Witchhazel			

See Appendix K for a detailed Shrub Replacement Plan

I focused the replacement process on the shrubs which appear at least 25 times as they provide the greatest water savings impact. Virginia and Henry Garnet's Sweetspire already appeared on site so I split the two species of Cherry Laurel between them. There is very little difference between the four species of Viburnums, so I replaced the least frequently occurring plants, Korean Spice and Shasta Doublefile, with the Arrowwood and Possumbaw.

Climate

Virginia has a humid and sub-tropical climate. Very hot and humid summers yet cool winters that often produce frost. There are four distinct seasons each year. During the summer months, short rain squalls are common, while the month of May receives the most precipitation. Annual average temperatures fall between 45 and 50 degrees Fahrenheit. The chart below shows the average monthly rainfall in Northern Virginia.

	Average Rainfall in Northern Virginia (inches)												
Jan	Jan Feb Mar Apr May Jun Jul Aug Sept Oct Nov Dec Year												
3.2 2.8 3.7 3.3 4.3 4.0 4.4 3.5 3.7 3.3 3.3 2.9 40.											40.6		

One of the factors in determining what kind of plant species will survive in a given area is the USDA Frost Zone map. This map classifies each zone, number 1-10, of the US based on minimum temperature and earliest and latest dates of possible frosting. This is called plant hardiness, and the plant hardiness for Middleburg, Virginia is Zone 6. Zone 6 plants have the ability to withstand temperature as low as -10 degrees F. The average date of the first frost is September 1 – September 30, while the average day of the last frost is between May 1 and May 30.



The lower the zone number, the higher the hardiness of the plant species. It is important to consider the optimal zone of each plant because it determines whether it will survive cold winters or hot summers. This is another reason why native plants should be considered in plantings because they are in the given zone and are able to survive the climate.

Current Irrigation System

The main supply for the currently proposed irrigation system comes from a pump located at an existing pond to the northwest of the building. The supply runs from that point, in a 3" PVC mainline, to several remote valves in each of the three areas. 1.5" to 3" PVC piping is used to distribute the water from the mainline to the irrigation fixtures. Immediately around the building, drip tubing is used for watering while the remaining portions have 4", 6" of 12" sprayhead fixtures. Quick couplers are attached to the main line that surrounds the culinary garden to allow for specialized hookups. A PVC sleeve is installed around the piping that goes underneath concrete walkways or driveways to prevent structural failure. A wireless rain sensor is located on the southeast side of the building to shut off the irrigation system if there has been enough rainfall.

Proposed Irrigation System

See Appendix L for full specifications

The proposed rainwater collection system is manufactured by Snyder Industries. This project will utilize a series of below ground cistern tanks in either the 1200 or 1700 gallon size. The tanks will be placed in series with one another to obtain the capacity required in each of the three irrigation areas seen below.



They will be placed in non traffic areas to reduce the possibility to structural failure. This system will tap into the current piping and fixtures and replace the pond pumping station. The location of tanks can be seen here:



The tanks have a pump inside that will be used to bring the rainwater to the surface. From there, the tanks are positioned such that the irrigation can mainly be gravity-fed.

Rainwater Collection Potential

The use of rainwater is an economical alternative to public water and is ideal for irrigation systems. This system will also reduce the water extraction from the onsite pond that is currently being tapped for the irrigation. The preservation of this pond during the hot summer months will enhance the appeal of the surrounding area. Rainwater is very low in minerals so it is an excellent source of irrigation for sensitive plants and flowers. It is not regulated by the municipality so in a time of drought, when water restrictions are in place, the rainwater collected from earlier months can protect your investments in landscaping. By utilizing rainwater collections tanks, storm water runoff is reduced, thus eliminating the danger of soil erosion, water drain overflow, and water pollution.

Rainwater can be collected from almost any surface, but bare rooftops provide the easiest collection and the water usually contains the least amount of contaminants and chemicals. Not all the water that strikes a rooftop can be collected because a portion is lost to evaporation, blowing wind, leaks, and overflowing gutters. The collectable water that can be obtained after the loss factors can be summarized in the following equation:

Collectable rainwater (gallons) = .5 x rainfall (inches) x area (square feet)

The Salamander Resort and Spa has approximately 28,600 square feet of roof area, which when calculated along with an average of 42.4 inches of rain annually comes out to 606,320 gallons/year of potential rainwater collection. As shown in the later analysis, not all of the potential collection is necessary.

Along the east coast of the US, the rainfall is relatively evenly distributed throughout the year which makes for easy rainwater collection and distribution. This also helps in simplifying the sizing of the water tanks as there is no need to worry about over sizing to account for drier months.

Water Requirements - MEP Breadth Analysis

Determining water use for an irrigation system depends heavily on the proportion of native plants present. According to Snyder Industries, temperate-climate plants, like the ones found in Northern Virginia, need about 1-inch of rainfall per week to survive. The plants in the local region obtain between 3.0 and 3.5 inches of rainfall a month, which translates to between .75 and

.88 inches a week. For my calculations, I will assume .75 inches per week. The factor of .6 is used in the following equation to represent water use of temperate-climate plants.

Gallons/week needed = .6 x (square feet)

Irrigation area 1, located to the north of the lodge is the largest of the three irrigation zones. The total area is $28,000 \text{ ft}^2$. The proposed irrigation system will be designed as a supplement to natural rainfall.

The above equation is used to determine the overall water requirement for this area.

 $.6 \times 28,000 \text{ ft}^2 = 16,800 \text{ gal/wk needed}$

In order to accurately size the rainwater collection tanks, the amount of natural rainfall in the irrigation area has to be determined. This calculation is done using the .75 inches per week assumption.

.75 in/week = $108 \text{ in}^3/\text{ft}^2$ = .47 gal/ft² .47 x 28,000 ft² = 13,160 gal/wk natural rainfall

The difference between the needed and natural rainfall, gallons per week is the determining factor for sizing the collection tank.

16,800 - 13,160 = 3,640 gal/wk

These same calculations are repeated for the two other irrigation areas and the findings are summarized in the following table:

Area	Square Footage	Needed (gal/wk)	Natural (gal/wk)	Difference (gal/wk)		
1	28,000	16,800	13,160	3,640		
2	12,825	7,700	6,030	1,670		
3	17,100	10,260	8,040	2,223		

The gallons/week difference that is needed for the proposed irrigation system must be matched up with a roof area that will adequately supply the cistern tanks. See Appendix M for the assigned roof collection area. The following table summarizes the cistern sizing:

Area	Rainwater Needed	Roof Area (ft^2)	Rainwater Collected (gal/wk)	Tanks	
1	3,640	9540	4220	(4) 1200 gal	
2	2 1,670 4831		2130	(2) 1200 gal	
3	2,223	8190	3620	(2)1700 gal	

I conservatively sized the tanks to err on the high side in order to account for droughts and dry spells. Area 3, on the east side of the building, is sized extra large because the garden will need additional manual watering.

Constructability and Schedule Impact

With the introduction of more native plant species on the project, it lessens the impact of the schedule. Native species are more readily found at local nurseries than the more exotic types. There are dozens of nurseries within 50 miles of Middleburg, Virginia that would be able to supply all the necessary native plants for the project. The larger and more exotic plants that the project requires, the longer the lead time is needed to get them to the site. Care should be taken to find a nearby nursery that can accommodate the size and scope of The Salamander Resort and Spa. It will probably be necessary to find numerous suppliers to fill the large and diverse order.

The Virginia Department of Conservation and Recreation gives information about the purchasing and selection of native plants. Due to the seasonal availability of many plants, ordering all the required plants at once can be quite difficult. Contact with the nurseries will be necessary to adequately gauge the amount of lead time required. Because of this problem, it is difficult to determine the impact on the project schedule. If the correct research is done, the project schedule has the potential to decrease in length because of the decrease in shipping distance.

A total of 8 cistern tanks will be placed in three different locations on site. Each set of tanks will require a concrete pad for the base. This can be performed when the foundations for the building are being placed, and will add one day to the schedule. In order to prevent the tank from floating in the soil, they must be strapped down with hooks cast into the concrete pad. It can only be backfilled once the tanks have been strapped down. The tanks are to be installed per Snyder Industries instructions.

In order to accommodate the additional rainwater, the gutters around the roof collection area will be increased by 2 inches. The main downspout leading to the tanks will be upgraded to a 6-inch pipe. The gutters and downspouts outside of the roof collection area will not change in size.

The fixtures and piping will not be changed so there is not a schedule impact. In place of the pond pump station and piping there will be three hook-ups to the cistern rainwater collection tanks. These activities have off setting durations so the schedule is not impacted.

Cost Analysis

The most significant additional cost is the price of the Snyder collection tanks. A summary of these costs is seen below:

Tank Size	Quantity	Price/each	Cost
1200 Gallons	6	\$2,900	\$17,400
1700 Gallons	2	\$3,600	\$7,200
		Total	\$24,600

The savings achieved from this proposal comes from the loss of the pond pump station, concrete pad, and subsequent piping to the irrigation system. The cost breakdown of the pump station is as follows:

Equipment	Price (\$)
Pump, Goulds 3656/Motor 3600 RPM	\$2,400
1 kVa Transformer	\$320
Variable Frequency Drive, ACS550	\$2,000
Exhasut Fan, 1320 CFM	\$80
Pressure Transducer	\$150
GB6 Electronic Controller, Tekleen	\$1,000
Backwash Filter	\$300
Total	\$6,250

The additional cost of this proposal, relative to the existing plan is \$18,350. This ignores some cost components in each system. For this rough estimate I am assuming the existing concrete pad, piping from the pump station to the system hookup cancel out the proposed cost of the cistern hookups, and concrete pads for the tanks. These costs would more or less cancel each other out and be relatively small in comparison to \$18,350.

The on-site pond is located quite a distance away from the building and is significantly lower in elevation. This poses two problems, first is the distance that the water needs to be pumped and second, the amount of electricity the pump uses to get it up the hill to the irrigation system. With the water collection system in place, the water tanks will be located directly underneath the irrigation areas. This will reduce the overall distance the water needs to travel from the source to

the irrigation system. Each tank will have a small pump that will distribute the rainwater. These motors only need to pump the water 3 vertical feet which will cut down on the energy use.

Conclusion and Recommendation

The use of native plants in landscaping can have a significant positive impact on water use without drastically changing the aesthetics. Native plants are accustomed to the climate and conditions of the location and are hardier and more likely to survive harsh conditions than that of non-native and exotic plants. It is important to note both the aesthetic characteristics and hardiness of a plant when replacing it with an alternative.

Some of the non-native species in the existing plan have close relatives that are native to the region. This made changing the plant species a trivial task and was confusing as to why the native plant was not specified in the first place. For the remaining non-native plants, focus was placed on the species that are used over 25 times.

The estimated additional cost of \$18,350 is a relatively small cost for increased efficiency and sustainability. It is recommended that The Salamander Resort and Spa implement the proposed rainwater collection system and native plant redesign.

Appendix A: Site Plan of Existing Conditions



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Appendix B: Site Layout Planning



Appendix C: D4 Cost Estimate

Tuesday, September 29, 2009

Statement of Probable Cost

Page 1

	Salamander	Resort and Spa - Mar	2009 - VA - Arlington	
	Prepared By: Paul Roberts		Prepared For: Paul Ro	berts
	Fax: Building Sq. Size: 229213 Bid Date: No. of floors: 4 No. of buildings: 1 Project Height: 1st Floor Height: 10.25 1st Floor Size:		Fax: Site Sq. Size: Building use: Foundation: CON Exterior Walls: STU Interior Walls: Roof Type: WOD Project Type: NEW	otel
Division		Percent	Sq. Cost	Amount
00	Bidding Requirements	7.22	9.60	2,200,317
	Bidding Requirements	7.22	9.60	2,200,317
01	General Requirements	3.75	4.99	1,144,345
	General Requirements	3.75	4.99	1,144,345
02	Site Work	4 08	5.42	1 242 343
01	Site Work	4.08	5.42	1,242,343
00	0 million and a	04.00	07.00	0.400.070
03	Concrete	21.00	27.92	6,400,073
04	Masonry	6.09	8.09	1,855,012
	Masonry	6.09	8.09	1,855,012
05	Metals	1.56	2.07	475,578
	Metals	1.56	2.07	475,578
06	Wood & Plastics	4.78	6.36	1,457,405
	Wood & Plastics	4.78	6.36	1,457,405
07	Thermal & Moisture Protection	1 01	2.54	581 807
07	Thermal & Moisture Protection	1.91	2.54	581,897
00	Denne 8 Miledana		0.50	4 9 17 4 99
08	Doors & Windows Doors & Windows	6.39 6.39	8.50 8.50	1,947,182 1,947,182
		0.00	0.00	1,011,102
09	Finishes	11.38	15.13	3,467,559
	Fillisties	11.30	15.15	3,467,559
10	Specialties	0.35	0.47	107,688
	Specialties	0.35	0.47	107,688
11	Equipment	0.09	0.12	26,443
	Equipment	0.09	0.12	26,443
12	Furnishings	0.05	0.06	13,973
	Furnishings	0.05	0.06	13,973
40	Constant Constant line	0.00	0.00	000 544
15	Special Construction	0.68	0.90	206,514
14	Conveying Systems	2.49	3.31	758,084
	Conveying Systems	2.49	5.51	100,004
15	Mechanical	19.61	26.08	5,978,605
	Mechanical	19.61	26.08	5,978,605
16	Electrical	8.60	11.43	2,620.616
	Electrical	8.60	11.43	2,620,616
Total Bui	Iding Costs	100.00	132.99	30,483,633

		Final Repo	rt 2010
Fuesday, September 29, 2009			Pagi
Total Non-Building Costs	100.00	0.00	0
Total Proiect Costs		30.483.	633

Appendix D: RS Means Reference Pages

COMMERCIAL/INDUSTRIAL/ M.350 Hotel, 4-7 Story											
								-			
osts per square foot	of floor are			75000	05000	115000	135000	155000	175000	195000	
	S.F. Area	35000	55000	75000	95000	420	722	754	783	850	
xterior Wall	L.F. Perimeter	314	401	497	555	039	142.70	161 70	160.15	159.60	
Pul II Counts	Steel Frame	182.95	173.80	169.95	166.40	164.80	103.70	101.70	158.20	157 60	
ace Brick with Concrete	R/Conc. Frame	180.95	171.85	167.95	164.40	162.85	161.75	159./5	156.20	15/ 05	
lock buckup	Steel Frame	177.25	168.95	165.40	162.35	160.90	159.90	158.20	156.85	150.35	
Glass and Metal	Steel Frame	175 50	167 25	163.70	160.60	159.15	158.10	156.50	155.15	154.60	
Curtain Walls	R/Conc. Frame	175.50	107.20	174.15	170.00	168.15	166.90	164.55	162.80	162.10	
Precast	Steel Frame	189.05	1/8.00	174.15	140.05	167.00	165.70	163.35	161.50	160.85	
Concrete Panels	R/Conc. Frame	188.20	177.55	1/3.10	100.05	107.00		0.15	1.05	1 75	
Perimeter Adi., Add or Deduct	Per 100 L.F.	9.50	6.05	4.40	3.50	2.85	2.45	2.15	1.75	1.25	
Story Hat, Adi., Add or Deduct	Per 1 Ft.	2.65	2.15	1.95	1.70	1.60	1.60	1.45	1.50	1.20	
	For B	asement, add	\$32.30 per s	quare foot of	basement ar	rea					
ne above costs were calculated using the basic esign alternatives and owner's requirements. R Common additives	specifications shown on t eported completed project	he facing page costs, for this	e. These costs type of structu	should be ad tre, range fro	djusted where m \$ 108.75	e necessary to to \$208.75	r ber S.F.		Unit	\$ Cost	
locarintian	Unit	\$ Cos	it	Description	1				01m		
ar, Front bar	LF.	360		Folde	rs, blankets &	sheets, king s	ize		Each	66,500	
Back bar	Lr. I F	202 - 3	75	Ironei	rs, 110" single	e roll	0.4		Each	12,200	
ooth, Upholstered, custom, straight	LF.	210-3	355	Com	ination wash	ner extractor 5	U#		Each	32,800	
Closed Circuit Surveillance, One station	Each	185	0	Sauna, Pr	efabricated,	complete			Each	5850	
Camera and monitor	Each	100	0	6' x -	4'				Each	6950	
For additional camera stations, add	2001	2,000		6' x	6' 0'				Each	8525	
30" x 20"	Each	595	0	0 X 8' v	8'				Each	10,10	
0.17 10/	Each	145	0	10'	x 12'				Each	14,00	
36" x 48"	cach	980	0	Smoke D	etectors				Fach	187	
36" × 48" Aluminum, 24" × 18"	Each		5	Ceili	ng type				Each	480	
36" × 48" Aluminum, 24" × 18" 48" × 32"	Each Each	202							600 TO 000		
30° × 48° Aluminum, 24° × 18° 48° × 32° 48° × 60°	Each Each	202		Duct	type						
36" x 48" Aluminum, 24" x 18" 48" x 32" 48" x 60" Elevators, Electric passenger, 5 stops 2500 economic	Each Each Each	202 167,2	200	Duct Sound Sy	type ystem	atts			Each	2350	
36° x 46" Aluminum, 24" x 18" 48" x 32" Elevators, Electric passenger, 5 stops 3500# capacity 5000# capacity	Each Each Each	202 167,1 170,1	200 700	Duct Sound Sy Amp Soor	type ystem blifier, 250 w	atts or wall			Each Each	2350 191	
30° x 48° Aluminum, 24" x 18" 48" x 60" Elevators, Electric passenger, 5 stops 3500# capacity 5000# capacity Additional stop, add	Each Each Each Each Each	202 167,1 170,1 13,6	200 700 600	Duct Sound Sy Amp Spe	type ystem blifier, 250 w aker, ceiling o Trumpet	atts or wall			Each Each Each	2350 191 365	
30° x 48° Aluminum, 24" x 18" 48" x 32" 48" x 60" Elevators, Electric passenger, 5 stops 3500# capacity 5000# capacity Additional stop, add Emergency Lighting, 25 wat, battery operated	Each Each Each Each Each	202 167,: 170,; 13,6	200 700 600	Duct Sound Sy Amp Spei	type ystem blifier, 250 we aker, ceiling o Trumpet nna, Master s	atts or wall system, 12 out	et		Each Each Outlet	2350 191 365 315	
30° x 48° Aluminum, 24° x 18° 48° x 32° 48° x 60° Elevators, Electric passenger, 5 stops 3500# capacity 5000# capacity Additional stop, add Emergency Lighting, 25 wort, battery operated Lead battery	Each Each Each Each Each Each	202 167,1 170,1 13,6 1 28	200 700 500 12	Duct Sound Sy Amp Sper TV Anter 30	type ystem olifier, 250 w aker, ceiling o Trumpet ma, Master s outlet	atts or wall system, 12 out	et		Each Each Outlet Outlet	2350 191 365 315 203	

. ...

Important: See the Reference Section for Location Factors

Locatio	n Factors						
STATE/ZIP	СПТҮ	Residential	Commercial	STATE/ZIP	CITY	Residential	Commercial
UTAH (CONT'd) 845 846-847 VERMONT	Price Provo	.70 .80	.78 .87	WYOMING (CON 823 824 825 826	T'd) Rawlins Worland Riverton Casper	.75 .74 .73 .76	.83 .81 .81
050 051 052 053 054 056	White River Jct. Bellows Falls Bennington Brattleboro Burlington Montpelier	.76 .78 .80 .80 .81 .82	.80 .82 .83 .84 .86 .84	827 828 829-831 CANADIAN FACTO	Newcastle Sheridan Rock Springs	.74 .79 .78 currency)	.81 .84 .83
057 058 059 VIRGINIA 220-221	Rutland St. Johnsbury Guildhall Fairfax	.81 .78 .77	.85 .80 .79 .93	ALBERTA	Calgary Edmonton Fort McMurray Lethbridge Lloydminster	1.14 1.13 1.14 1.11 1.06	1.14 1.14 1.13 1.09 1.05
222 223 224-225 226 227 228	Arlington Alexandria Fredericksburg Winchester Culpeper Harrisonburg	1.03 1.07 .94 .91 .99 .89	.93 .95 .88 .86 .88 .86	BRITISH COLUMB	Medicine Hat Red Deer IA Kamloops Prince George	1.07 1.07 1.05 1.05	1.05 1.05 1.06 1.07
229 230-232 233-235 236 237 238	Charlottesville Richmond Norfolk Newport News Portsmouth Petersburg	.90 .98 1.00 .99 .92 .96	.86 .88 .89 .88 .86 .86	MANITOBA	Vancouver Victoria Brandon Portage la Prairie	1.06 .99 1.02	1.11 1.02
239 240-241 242 243 244 245 246	Farmville Roanoke Bristol Pulaski Staunton Lynchburg Grundy	.88 .97 .85 .83 .90 .95 .83	.81 .85 .81 .80 .84 .86 .80	NEW BRUNSWICK	Bathurst Dalhousie Fredericton	.94 .94 1.01 95	.95 .95 .98 .96
VASHINGTON 80-981,987 82 83-984 85	Seattle Everett Tacoma	1.02 1.04 1.02	1.04 1.02 1.03	NEWFOUNDLAND	Newcastle St. John Corner Brook	.94 1.01	.95 .98
86 88 89 90-992 93	Vancouver Wenatchee Yakima Spokane Richland	.97 .92 .96 .99 .97	1.02 1.01 .95 .98 .95 .95	NORTHWEST TERF	RITORIES Yellowknife	.98	1.06
94 IEST VIRGINIA 47-248 49 50-253 54 55-257	Bluefield Lewisburg Charleston Martinsburg Huntington	.96 .88 .90 .95 .86 .96	.94 .89 .92 .95 .90 .90	<u>s</u> .	Bridgewater Dartmouth Halifax New Glasgow Sydney Truro Yarmouth	.97 .98 1.00 .97 .96 .97 .97	.99 1.00 1.02 .99 .97 .99 .99
58-259 50 52 53-264 55 56 57 58	Beckley Wheeling Parkersburg Buckhannon Clarksburg Morgantown Gassaway Romney Petersburg	.90 .92 .91 .91 .91 .91 .92 .91 .89 .91	.93 .96 .95 .95 .95 .95 .95 .95 .95 .95 .92 .93	ONTARIO	Barrie Brantford Cornwall Hamilton Kingston Kitchener London North Bay	1.13 1.14 1.14 1.16 1.14 1.09 1.14 1.11	1.08 1.09 1.08 1.12 1.09 1.05 1.10 1.07
ISCONSIN 30,532 31 44 15 17 17 18 9 0 1-543 4	Milwaukee Kenosha Racine Beloit Madison Lancaster Portage New Richmond Green Bay Wausau	1.07 1.03 1.02 .98 .98 .97 .96 .99 1.00 .94	1.03 1.00 .97 .98 .94 .95 .95 .96 .92		Usriawa Ottawa Owen Sound Peterborough Sarria Sault Ste Marie St. Catharines Sudbury Thunder Bay Timmins Toronto Windsor	1.13 1.16 1.11 1.12 1.14 1.07 1.10 1.07 1.12 1.11 1.17 1.11	1.08 1.11 1.08 1.09 1.04 1.05 1.04 1.05 1.07 1.14 1.05
5 6 7 8 9	Rhinelander La Crosse Eau Claire Superior Oshkosh	.94 .94 .97 .98 .94	.94 .94 .95 .96 .93	PRINCE EDWARD IS	LAND Charlottetown Summerside	.92 .92	.95 .95
OMING 0 1 2	Cheyenne Yellowstone Nat. Pk. Wheatland	.82 .74 .74	.86 .81 .82		Cap-de-la-Madeleine Charlesbourg Chicoutimi Gatineau	1.13 1.13 1.16 1.12	1.04 1.04 1.05 1.03

Final Report 2010

Appendix E: Detailed Structural Systems Estimate

Concrete Columns:

	Normal Weight Concrete, 3000 psi											
Location	Size	Quantity	Total CY	Ur	nit Mat'l Cost	Material Cost	Total Cost					
Area 1	18 x 18	9	7.50	\$	101.00	\$ 757.48	\$ 757.48					
	24 x 24	40	59.26	\$	101.00	\$ 5,985.02	\$ 5,985.02					
Area 2	18 x 18	22	18.33	\$	101.00	\$ 1,851.61	\$ 1,851.61					
	24 x 24	31	45.92	\$	101.00	\$ 4,638.39	\$ 4,638.39					
	24 x 72	2	8.89	\$	101.00	\$ 897.75	\$ 897.75					
Area 3	12 x 12	6	2.22	\$	101.00	\$ 224.44	\$ 224.44					
	12 x 16	2	0.99	\$	101.00	\$ 99.75	\$ 99.75					
	18 x 18	21	17.50	\$	101.00	\$1,767.45	\$ 1,767.45					
	18 x 36	5	8.33	\$	101.00	\$ 841.64	\$ 841.64					
	24 x 24	7	10.37	\$	101.00	\$1,047.38	\$ 1,047.38					
	26 x 26	5	8.69	\$	101.00	\$ 878.01	\$ 878.01					
Area 4	10 x 30	4	3.09	\$	101.00	\$ 311.72	\$ 311.72					
	12 x 12	11	4.07	\$	101.00	\$ 411.47	\$ 411.47					
	12 x 24	8	5.93	\$	101.00	\$ 598.50	\$ 598.50					
	16 x 24	2	1.98	\$	101.00	\$ 199.50	\$ 199.50					
	16 x 28	73	84.11	\$	101.00	\$ 8,495.40	\$ 8,495.40					
						Total	\$29,005.51					

	Placing Concrete, pumped											
Location	Size	Quantity	Total CY	Un	it Labor Cost	Lal	bor Cost	Eq	Unit uipment	Equipment Cost	Total Cost	
Area 1	18 x 18	9	7.50	\$	24.00	\$	179.99	\$	8.80	\$ 66.00	\$ 245.99	
	24 x 24	40	59.26	\$	23.50	\$ 1	L,392.55	\$	8.60	\$ 509.62	\$1,902.17	
Area 2	18 x 18	22	18.33	\$	24.00	\$	439.99	\$	8.80	\$ 161.33	\$ 601.32	
	24 x 24	31	45.92	\$	23.50	\$1	L,079.23	\$	8.60	\$ 394.95	\$1,474.18	
	24 x 72	2	8.89	\$	15.50	\$	137.77	\$	5.65	\$ 50.22	\$ 187.99	
Area 3	12 x 12	6	2.22	\$	36.00	\$	80.00	\$	13.15	\$ 29.22	\$ 109.22	
	12 x 16	2	0.99	\$	24.00	\$	23.70	\$	8.80	\$ 8.69	\$ 32.39	
	18 x 18	21	17.50	\$	24.00	\$	419.99	\$	8.80	\$ 154.00	\$ 573.98	
	18 x 36	5	8.33	\$	15.50	\$	129.16	\$	5.65	\$ 47.08	\$ 176.25	
	24 x 24	7	10.37	\$	23.50	\$	243.70	\$	8.60	\$ 89.18	\$ 332.88	
	26 x 26	5	8.69	\$	15.50	\$	134.74	\$	5.65	\$ 49.12	\$ 183.86	
Area 4	10 x 30	4	3.09	\$	23.50	\$	72.53	\$	8.60	\$ 26.54	\$ 99.07	
	12 x 12	11	4.07	\$	36.00	\$	146.66	\$	13.15	\$ 53.57	\$ 200.24	
	12 x 24	8	5.93	\$	23.50	\$	139.26	\$	8.60	\$ 50.96	\$ 190.22	
	16 x 24	2	1.98	\$	23.50	\$	46.42	\$	8.60	\$ 16.99	\$ 63.41	
	16 x 28	73	84.11	\$	15.50	\$ 1	L,303.75	\$	5.65	\$ 475.24	\$1,778.99	
										Total	\$8,152.15	

	Forms in Place, plywood 2 use														
Location	Sizo	Quantity	SECA	Un	it Mat'l	M	aterial	Un	it Labor	Labor	То	tal Cost			
LUCATION	5120	Quantity	SFCA		Cost		Cost		Cost	Cost	10	Lai Cust			
Area 1	18 x 18	9	26.00	\$	1.37	\$	35.62	\$	5.60	\$ 145.60	\$	181.22			
	24 x 24	40	28.00	\$	1.37	\$	38.36	\$	5.60	\$ 156.80	\$	195.16			
Area 2	18 x 18	22	26.00	\$ 1.37		\$	35.62	\$	5.60	\$ 145.60	\$	181.22			
	24 x 24	31	28.00	\$	1.37	\$	38.36	\$	5.60	\$ 156.80	\$	195.16			
	24 x 72	2	36.00	\$	1.03	\$	37.08	\$	6.28	\$ 226.08	\$	263.16			
Area 3	12 x 12	6	24.00	\$	1.25	\$	30.00	\$	5.75	\$ 138.00	\$	168.00			
	12 x 16	2	24.67	\$	1.19	\$	29.35	\$	5.65	\$ 139.37	\$	168.72			
	18 x 18	21	26.00	\$	1.37	\$	35.62	\$	5.60	\$ 145.60	\$	181.22			
	18 x 36	5	29.00	\$	1.03	\$	29.87	\$	6.28	\$ 182.12	\$	211.99			
	24 x 24	7	28.00	\$	1.37	\$	38.36	\$	5.60	\$ 156.80	\$	195.16			
	26 x 26	5	28.67	\$	1.03	\$	29.53	\$	6.28	\$ 180.03	\$	209.55			
Area 4	10 x 30	4	26.67	\$	1.03	\$	27.47	\$	6.28	\$ 167.47	\$	194.93			
	12 x 12	11	24.00	\$	1.25	\$	30.00	\$	5.75	\$ 138.00	\$	168.00			
	12 x 24 8 26.00		\$	1.37	\$	35.62	\$	5.60	\$ 145.60	\$	181.22				
	16 x 24 2 26.67					\$	36.53	\$	5.60	\$ 149.33	\$	185.87			
16 x 28 73 27.33 \$ 1.03 \$ 28.15 \$ 6.28 \$ 171.65											\$	199.81			
										Total	\$3	,080.39			

Concrete Slabs:

	Normal Weight Concrete														
Location	Type of Slab	Area (SF)	Depth (in)	PSI	Total CY	Unit Mat'l Cost	Material Cost	Total Cost							
Area 1	SOG 20500 5 3000 43.94 \$ 101.00 \$ 4,437.80														
Area 2	SOG 20400 5 3000 43.72 \$ 101.00 \$ 4,416.15														
Area 3	SOG	17100	5	3000	36.65	\$ 101.00	\$ 3,701.77	\$ 3,701.77							
Area 4.B	SOG	30000	5	3000	64.30	\$ 104.00	\$ 6,687.24	\$ 6,687.24							
Area 4.1	Elevated	30000	9	3500	208.33	\$ 104.00	\$21,666.64	\$ 21,666.64							
Area 4.2	Elevated	30000	9	3500	208.33	\$ 104.00	\$21,666.64	\$ 21,666.64							
Area 4.3	Elevated	30000	9	3500	208.33	\$ 104.00	\$21,666.64	\$ 21,666.64							
Area 4.P	Elevated	30000	10	3500	257.20	\$ 104.00	\$26,748.94	\$ 26,748.94							
							Total	\$110,991.83							

Placing, pumped														
Location	Location Type of Slab Depth (in) Total CY Unit Labor Cost Labor Cost Unit Equipment Equipment													
Area 1	SOG	5	6.10	\$ 268.03	\$	1,001.80								
Area 2	SOG	5	43.72	730.19	\$	6.10	\$ 266.72	\$	996.91					
Area 3	SOG	5	36.65	16.7	612.08	\$	6.10	\$ 223.57	\$	835.65				
Area 4.B	SOG	5	64.30	16.7	1073.82	\$	6.10	\$ 392.23	\$	1,466.05				
Area 4.1	Elevated	9	208.33	13.55	2822.91	\$	4.94	\$ 1,029.17	\$	3,852.08				
Area 4.2	Elevated	9	208.33	13.55	2822.91	\$	4.94	\$ 1,029.17	\$	3,852.08				
Area 4.3	Elevated	9	208.33	13.55	2822.91	\$	4.94	\$ 1,029.17	\$	3,852.08				
Area 4.P	Elevated	10	257.20	13.55	3485.08	\$	4.94	\$ 1,270.57	\$	4,755.65				
Total \$														

Forming														
Location Type of Slab SFCA Depth (in) L.F. Unit Mat'l Material Unit Labor Cost Cost Cost Cost Cost Cost Cost Cost														
Area 1	SOG	295.83	5	710.00	\$	0.46	\$	326.60	\$	3.03	\$	2,151.30	\$ 2,477.90	
Area 2 SOG 254.17 5 610.00 \$ 0.46 \$ 280.60 \$ 3.03												1,848.30	\$ 2,128.90	
Area 3	SOG	291.67	5	700.00	\$	0.46	\$	322.00	\$	3.03	\$	2,121.00	\$ 2,443.00	
Area 4.B	SOG	500.00	5	1200.00	\$	0.46	\$	552.00	\$	3.03	\$	3,636.00	\$ 4,188.00	
Area 4.1	Elevated	900.00	9	1200.00	\$	0.70	\$	840.00	\$	5.90	\$	7,080.00	\$ 7,920.00	
Area 4.2	Elevated	900.00	9	1200.00	\$	0.70	\$	840.00	\$	5.90	\$	7,080.00	\$ 7,920.00	
Area 4.3	Elevated	900.00	9	1200.00	\$	0.70	\$	840.00	\$	5.90	\$	7,080.00	\$ 7,920.00	
Area 4.P	Elevated	1000.00	10	1200.00	\$	0.70	\$	840.00	\$	5.90	\$	7,080.00	\$ 7,920.00	
											Tot	al	\$42,917.80	

Footings

	Normal Weight Concrete, 3000 psi													
Width (ft.)	Length (ft.)	Depth (in.)	Quantity	Total CY	Unit Mat'l Cost	Material Cost	Total Cost							
4	4	12	4	2.37	\$ 101.00	\$ 239.41	\$ 239.41							
4.5	4.5	12	9	6.75	\$ 101.00	\$ 681.75	\$ 681.75							
5	5	12	8	7.41	\$ 101.00	\$ 748.15	\$ 748.15							
5.5	5.5	13	17	20.63	\$ 101.00	\$ 2,083.98	\$ 2,083.98							
6	6	14	42	65.33	\$ 101.00	\$ 6,598.66	\$ 6,598.66							
6.5	6.5	16	17	35.47	\$ 101.00	\$ 3,582.38	\$ 3,582.38							
7	7	17	24	61.70	\$ 101.00	\$ 6,232.07	\$ 6,232.07							
7.5	7.5	18	31	96.87	\$ 101.00	\$ 9,784.37	\$ 9,784.37							
8	8	19	10	37.53	\$ 101.00	\$ 3,790.61	\$ 3,790.61							
8.5	8 19 10 8.5 20 11		49.06	\$ 101.00	\$ 4,954.92	\$ 4,954.92								
9	9	21	2	10.50	\$ 101.00	\$ 1,060.50	\$ 1,060.50							
9.5	9.5	22	10	61.28	\$ 101.00	\$ 6,189.36	\$ 6,189.36							
10.5	10.5	25	12	102.08	\$ 101.00	\$10,310.41	\$ 10,310.41							
12	12	28	1	12.44	\$ 101.00	\$ 1,256.89	\$ 1,256.89							
12.5	12.6	28	12	163.33	\$ 101.00	\$16,496.65	\$ 16,496.65							
13	13	30	12	187.78	\$ 101.00	\$18,965.54	\$ 18,965.54							
14	14	31	10	187.53	\$ 101.00	\$18,940.60	\$ 18,940.60							
15	15	34	1	23.61	\$ 101.00	\$ 2,384.72	\$ 2,384.72							
9	12	19	2	12.67	\$ 101.00	\$ 1,279.33	\$ 1,279.33							
10	14	26	1	11.23	\$ 101.00	\$ 1,134.69	\$ 1,134.69							
10	15	24	3	33.33	\$101.00	\$ 3,366.66	\$ 3,366.66							
18.5	28	24	1	38.37	\$101.00	\$ 3,875.40	\$ 3,875.40							
5	8	16	1	1.98	\$101.00	\$ 199.51	\$ 199.51							
						Total	\$124,156.54							

				Placing	Со	ncrete	Fc	ootings,	Pur	nped				
Width (ft.)	Length (ft.)	Depth (in.)	Quantity	Total CY	Uni	t Labor Cost	La	bor Cost	E	Unit quipment	Eq	uipment Cost	Т	otal Cost
4	4	12	4	2.37	\$	14.45	\$	34.25	\$	5.25	\$	12.44	\$	46.70
4.5	4.5	12	9	6.75	\$	14.45	\$	97.54	\$	5.25	\$	35.44	\$	132.97
5	5	12	8	7.41	\$	14.45	\$	107.04	\$	5.25	\$	38.89	\$	145.93
5.5	5.5	13	17	20.63	\$	14.45	\$	298.15	\$	5.25	\$	108.33	\$	406.48
6	6	14	42	65.33	\$	14.45	\$	944.07	\$	5.25	\$	343.00	\$	1,287.07
6.5	6.5	16	17	35.47	\$	14.45	\$	512.53	\$	5.25	\$	186.21	\$	698.74
7	7	17	24	61.70	\$	14.45	\$	891.62	\$	5.25	\$	323.94	\$	1,215.56
7.5	7.5	18	31	96.87	\$	14.45	\$	1,399.84	\$	5.25	\$	508.59	\$	1,908.44
8	8	19	10	37.53	\$	14.45	\$	542.32	\$	5.25	\$	197.04	\$	739.36
8.5	8.5	20	11	49.06	\$	14.45	\$	708.90	\$	5.25	\$	257.56	\$	966.45
9	9	21	2	10.50	\$	14.45	\$	151.72	\$	5.25	\$	55.12	\$	206.85
9.5	9.5	22	10	61.28	\$	14.45	\$	885.51	\$	5.25	\$	321.72	\$	1,207.23
10.5	10.5	25	12	102.08	\$	14.45	\$	1,475.10	\$	5.25	\$	535.94	\$	2,011.04
12	12	28	1	12.44	\$	14.45	\$	179.82	\$	5.25	\$	65.33	\$	245.16
12.5	12.6	28	12	163.33	\$	14.45	\$	2,360.16	\$	5.25	\$	857.50	\$	3,217.66
13	13	30	12	187.78	\$	14.45	\$	2,713.39	\$	5.25	\$	985.83	\$	3,699.22
14	14	31	10	187.53	\$	14.45	\$	2,709.82	\$	5.25	\$	984.54	\$	3,694.35
15	15	34	1	23.61	\$	14.45	\$	341.18	\$	5.25	\$	123.96	\$	465.14
9	12	19	2	12.67	\$	14.45	\$	183.03	\$	5.25	\$	66.50	\$	249.53
10	14	26	1	11.23	\$	14.45	\$	162.34	\$	5.25	\$	58.98	\$	221.32
10	15	24	3	33.33	\$	14.45	\$	481.67	\$	5.25	\$	175.00	\$	656.67
18.5	28	24	1	38.37	\$	14.45	\$	554.45	\$	5.25	\$	201.44	\$	755.90
5	8	16	1	1.98	\$	14.45	\$	28.54	\$	5.25	\$	10.37	\$	38.91
											Tot	al 🗌	\$2	24,216.67

Forms in Place, Plywood, 2 use

Width (ft.)	Length (ft.)	Depth (in.)	Quantity	SFCA	Unit Mat'l Cost		Material Cost	Unit Labor Cost	Labor Cost	Total Cost
4	4	12	4	72.00	\$	4.10	\$ 295.20	\$ 2.75	\$ 198.00	\$ 493.20
4.5	4.5	12	9	180.00	\$	4.10	\$ 738.00	\$ 2.75	\$ 495.00	\$ 1,233.00
5	5	12	8	176.00	\$	4.10	\$ 721.60	\$ 2.75	\$ 484.00	\$ 1,205.60
5.5	5.5	13	17	410.83	\$	4.10	\$ 1,684.42	\$ 2.75	\$1,129.79	\$ 2,814.21
6	6	14	42	1106.00	\$	4.10	\$ 4,534.60	\$ 2.75	\$3,041.50	\$ 7,576.10
6.5	6.5	16	17	487.33	\$	4.10	\$ 1,998.07	\$ 2.75	\$1,340.17	\$ 3,338.23
7	7	17	24	740.00	\$	4.10	\$ 3,034.00	\$ 2.75	\$2,035.00	\$ 5,069.00
7.5	7.5	18	31	1023.00	\$	4.10	\$ 4,194.30	\$ 2.75	\$2,813.25	\$ 7,007.55
8	8	19	10	351.67	\$	4.10	\$ 1,441.83	\$ 2.75	\$ 967.08	\$ 2,408.92
8.5	8.5	20	11	410.67	\$	4.10	\$ 1,683.73	\$ 2.75	\$1,129.33	\$ 2,813.07
9	9	21	2	79.00	\$	4.10	\$ 323.90	\$ 2.75	\$ 217.25	\$ 541.15
9.5	9.5	22	10	416.67	\$	4.10	\$ 1,708.33	\$ 2.75	\$1,145.83	\$ 2,854.17
10.5	10.5	25	12	554.00	\$	4.10	\$ 2,271.40	\$ 2.75	\$1,523.50	\$ 3,794.90
12	12	28	1	52.67	\$	4.10	\$ 215.93	\$ 2.75	\$ 144.83	\$ 360.77
12.5	12.6	28	12	658.40	\$	4.10	\$ 2,699.44	\$ 2.75	\$1,810.60	\$ 4,510.04
13	13	30	12	684.00	\$	4.10	\$ 2,804.40	\$ 2.75	\$1,881.00	\$ 4,685.40
14	14	31	10	611.67	\$	4.10	\$ 2,507.83	\$ 2.75	\$1,682.08	\$ 4,189.92
15	15	34	1	65.67	\$	4.10	\$ 269.23	\$ 2.75	\$ 180.58	\$ 449.82
9	12	19	2	90.33	\$	4.10	\$ 370.37	\$ 2.75	\$ 248.42	\$ 618.78
10	14	26	1	52.33	\$	4.10	\$ 214.57	\$ 2.75	\$ 143.92	\$ 358.48
10	15	24	3	162.00	\$	4.10	\$ 664.20	\$ 2.75	\$ 445.50	\$ 1,109.70
18.5	28	24	1	97.00	\$	4.10	\$ 397.70	\$ 2.75	\$ 266.75	\$ 664.45
5	8	16	1	28.67	\$	4.10	\$ 117.53	\$ 2.75	\$ 78.83	\$ 196.37
									Total	\$58,292.82

Concrete Beams:	
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		Normal	Weight	Concrete	3000 psi		
Size	Length Range (ft.)	Avg. Length (ft.)	Quantity	Total CY	Unit Mat'l Cost	Material Cost	Total Cost
12 x 24							
16 x 24	10.15	12.5	1	0.93	\$ 101.00	\$ 93.52	\$ 93.52
10 x 24	5 10	75	1	0.74	\$ 101 00	\$ 74.81	\$ 74.81
	10.15	12.5	2	2.47	\$ 101.00	\$ 249.38	\$ 249.38
18 x 22							-
	0.5	7.5	1	0.76	\$ 101.00	\$ 77.15	\$ 77.15
18 x 32	10 15	12.5	1	1.95	¢ 101 00	¢ 197.04	¢ 197.04
22 x 24	10.15	12.5		1.85	\$ 101.00	\$ 107.04	\$ 187.04
	5.10	7.5	1	1.02	\$ 101.00	\$ 102.87	\$ 102.87
24 x 24							
	0.5	7.5	2	2.22	\$ 101.00	\$ 224.44	\$ 224.44
	5.10	7.5	2	2.22	<u>\$ 101.00</u>	\$ 224.44	\$ 224.44
	10.15	12.5	2	3.70 7.78	\$ 101.00	\$ 374.07	\$ 374.07
	20.25	22.5	10	33.33	\$ 101.00	\$3,366.66	\$ 3,366,66
	25.30	27.5	5	20.37	\$ 101.00	\$2,057.41	\$ 2,057.41
24 x 30			-				
	10.15	12.5	1	2.31	\$ 101.00	\$ 233.80	\$ 233.80
	15.20	17.5		3.24	\$ 101.00 \$ 101.00	\$ <u>327.31</u>	\$ <u>327.31</u> ¢ 1.683.33
	25.30	27.5	2	10.19	\$ 101.00	\$1,028.70	\$ 1.028.70
24 x 32					+	+=/======	<u> </u>
	10.15	12.5	1	2.47	\$ 101.00	\$ 249.38	\$ 249.38
24 x 57		00 F				+ 700 50	+ 700 50
	20.25	22.5	1	7.92	\$ 101.00	\$ 799.58	\$ 799.58
12 x 18	23.30	27.5	L	9.08	\$ 101.00	\$ 977.27	ъ <i>911.21</i>
	5.10	7.5	4	1.67	\$ 101.00	\$ 168.33	\$ 168.33
12 x 24						• •	•
	10.15	12.5	13	12.04	\$ 101.00	\$1,215.74	\$ 1,215.74
	15.20	17.5	6	7.78	\$ 101.00	\$ 785.55	\$ 785.55
12 x 36	20.25	22.5	0	10.00	\$ 101.00	\$1,010.00	\$ 1,010.00
	10.15	12.5	4	5.56	\$ 101.00	\$ 561.11	\$ 561.11
	15.20	17.5	2	3.89	\$ 101.00	\$ 392.78	\$ 392.78
10.00	15.20	17.5	2	3.89	\$ 101.00	\$ 392.78	\$ 392.78
12 x 38	10 15	12 5	1	1 47	¢ 101 00	¢ 149.07	¢ 149.07
	10.15	12.5	Ł	1.47	\$ 101.00	\$ 140.07	<u></u> \$ 140.07
		F	Post-Tensi	oning Bear	ns	-	-
10 22							
18 x 22		19	8	15.48	¢ 101 00	¢1 563 63	¢ 1 563 63
		30	2	6.11	\$ 101.00	\$ 617.22	\$ 617.22
		40	8	32.59	\$ 101.00	\$3,291.85	\$ 3,291.85
		50	9	45.83	\$ 101.00	\$4,629.16	\$ 4,629.16
18 x 25		E0		F 70	¢ 101.00	# 504 40	¢ E04.40
22 x 24		50		5.79	⇒ 101.00	⇒ 584.49	\$
		30	1	4.07	\$ 101.00	\$ 411.48	\$ 411.48
		40	1	5.43	\$ 101.00	\$ 548.64	\$ 548.64
22 x 32						+ 66 + 5=	+
		19	2	6.88	\$ 101.00	\$ 694.95	\$ 694.95
		<u>40</u> 50	<u>∠</u> 1	9,05	$\frac{3}{101.00}$	\$ 914 40	$\frac{3}{5}$ 1,463.04 \$ 914.40
22 x 36				5.05	<i>¥</i> 101.00	¥ 514.40	
		19	1	3.87	\$ 101.00	\$ 390.91	\$ 390.91
		40	1	8.15	\$ 101.00	\$ 822.96	\$ 822.96
24 26		50	1	10.19	\$ 101.00	\$1,028.70	\$ 1,028.70
24 X 36	10 15	12.5	1	2 78	\$ 101 00	\$ 280 56	\$ 280 56
	20.25	22.5	2	10.00	\$ 101.00	\$1,010.00	\$ 1,010.00
	25.30	27.5	2	12.22	\$ 101.00	\$1,234.44	\$ 1,234.44
						Total	\$36,043.09

			Plac	cing Con	crete, P	umped				
Size	Length Range (ft.)	Avg. Length (ft.)	Quantity	Total CY	Unit Labor Cost	Labor Cost	Unit Equipment Cost	Equipment Cost	Total C	Cost
12 x 24	10.15	12.5	1	0.93	\$36.00	\$ 33.33	\$ 13.15	\$ 12.18	\$ 4	5.51
16 x 24	5.10	7.5	1	0.74	\$36.00	\$ 26.67	\$ 13.15	\$ 9.74	\$ 3	6.41
10 2 22	10.15	12.5	2	2.47	\$36.00	\$ 88.89	\$ 13.15	\$ 32.47	\$ 12	1.36
10 X 22	0.5	7.5	1	0.76	\$36.00	\$ 27.50	\$ 13.15	\$ 10.05	\$ 3	7.55
18 x 32	10.15	12.5	1	1.85	\$36.00	\$ 66.67	\$ 13.15	\$ 24.35	\$ 9	1.02
22 x 24	F 10	7 5	-	1.02	+ 2C 00	+ 26.67	+ 10.1E	+ 12.20	+ -	0.00
24 x 24	5.10	7.5	L	1.02	\$36.00	\$ 30.07	\$ 13.15	\$ 13.39	\$ D	0.06
	0.5	7.5	2	2.22	\$36.00	\$ 80.00 \$ 80.00	\$ 13.15 ¢ 13.15	\$ 29.22 \$ 29.22	\$ 10	9.22
	10.15	12.5	2	3.70	\$36.00	\$ 133.33	\$ 13.15	\$ 48.70	\$ 10 \$ 18	2.04
	15.20	17.5	3	7.78	\$36.00	\$ 280.00	\$ 13.15	\$ 102.28	\$ 38	2.28
	20.25	22.5	10	33.33	\$36.00	\$1,200.00	\$ 13.15	\$ 438.33	\$ 1,63	8.33
24 x 30	25.30	27.5	5	20.37	\$30.00	\$ 733.33	\$ 13.15	\$ 207.87	\$ 1,00	1.20
	10.15	12.5	1	2.31	\$36.00	\$ 83.33	\$ 13.15	\$ 30.44	\$ 11	3.77
	15.20	17.5	1	3.24	\$36.00	\$ 116.67	\$ 13.15 ¢ 12.15	\$ 42.62	\$ 15	9.28
	25.30	27.5	2	10.07	\$36.00	\$ 366.67	\$ 13.15	\$ 133.94	\$ 50	0.60
24 x 32						•			•	
	10.15	12.5	1	2.47	\$36.00	\$ 88.89	\$ 13.15	\$ 32.47	\$ 12	1.36
24 X 37	20.25	22.5	1	7.92	\$36.00	\$ 285.00	\$ 13.15	\$ 104.10	\$ 38	9.10
	25.30	27.5	1	9.68	\$36.00	\$ 348.33	\$ 13.15	\$ 127.24	\$ 47	5.57
12 x 18	F 10	7 5	4	1.67	+26.00	+ 60.00	+ 10.15	± 21.02	+ 0	1 0 0
12 x 24	5.10	7.5	4	1.67	\$30.00	\$ 60.00	\$ 15.15	\$ 21.92	<u> </u>	1.92
	10.15	12.5	13	12.04	\$36.00	\$ 433.33	\$ 13.15	\$ 158.29	\$ 59	1.62
	15.20	17.5	6	7.78	\$36.00	\$ 280.00	\$ 13.15	\$ 102.28	\$ 38	2.28
12 x 36	20.25	22.5	0	10.00	\$30.00	\$ 300.00	\$ 15.15	\$ 131.50	\$ 49	1.50
	10.15	12.5	4	5.56	\$36.00	\$ 200.00	\$ 13.15	\$ 73.06	\$ 27	3.06
	15.20	17.5	2	3.89	\$36.00	\$ 140.00	\$ 13.15	\$ 51.14	\$ 19	1.14
12 x 38	15.20	17.5	2	3.89	\$36.00	\$ 140.00	\$ 13.15	\$ 51.14	\$ 19	1.14
12 × 50	10.15	12.5	1	1.47	\$36.00	\$ 52.78	\$ 13.15	\$ 19.28	\$ 7	2.06
				Post-Tens	ionina Bez	ams				
				1 000 1 0110	Joining Doo					
18 x 22		10	0	15 40	+ 2C 00	+ FF7 22	+ 10.1F	¢ 202 F0	+ 70	0.01
		30	8	15.48 6.11	\$36.00	\$ 220.00	\$ 13.15	\$ 203.58	\$ 76	0.91
		40	8	32.59	\$36.00	\$1,173.33	\$ 13.15	\$ 428.59	\$ 1,60	1.92
10 25		50	9	45.83	\$36.00	\$1,650.00	\$ 13.15	\$ 602.71	\$ 2,25	2.71
10 X 25		50	1	5.79	\$36.00	\$ 208.33	\$ 13.15	\$ 76.10	\$ 28	4.43
22 x 24			_							-
		30	1	4.07	\$36.00	\$ 146.67	\$ 13.15	\$ 53.57	\$ 20	0.24
22 x 32		40	L	5.43	\$30.00	9 193.20		\$ 71.43	ə 26	0.99
		19	2	6.88	\$36.00	\$ 247.70	\$ 13.15	\$ 90.48	\$ 33	8.18
		40	2	14.49	\$36.00	\$ 521.48	\$ 13.15	\$ 190.49	\$ 71	1.97
22 x 36		50		9.05	\$30.00	⇒ 325.93	\$ 13.15	\$ 119.05	<u></u> ≯ 44	4.98
		19	1	3.87	\$36.00	\$ 139.33	\$ 13.15	\$ 50.90	\$ 19	0.23
		40	1	8.15	\$36.00	\$ 293.33	\$ 13.15	\$ 107.15	\$ 40	0.48
24 x 36		50	T	10.19	\$36.00	\$ 306.6/	\$ 13.15	\$ 133.94	\$ 50	0.60
2	10.15	12.5	1	2.78	\$36.00	\$ 100.00	\$ 13.15	\$ 36.53	<u>\$ 1</u> 3	6.53
	20.25	22.5	2	10.00	\$36.00	\$ 360.00	\$ 13.15	\$ 131.50	\$ 49	1.50
	23.30	27.5	Ζ	12.22	\$30.00	\$ 440.00	\$ 13.15	\$ 160.72 Total		0.72

			Forms	s in Pla	ace,	Plywo	bod	, 2 use					
Size	Length Range (ft.)	Avg. Length (ft.)	Quantity	SFCA	Uni C	t Mat'l Cost	Μ	laterial Cost	Unit (: Labor Cost	Labor Cost	То	tal Cost
12 x 24	10.15	10.5		21.00	L +		-	10.67		E 10	+ 1 5 0 1 0	+	206 77
16 x 24	10.15	12.5	1	31.00	\$	1.57	\$	48.67	\$	5.10	\$158.10	\$	206.77
10 / 24	5.10	7.5	1	21.67	\$	1.57	\$	34.02	\$	5.10	\$110.50	\$	144.52
	10.15	12.5	2	31.67	\$	1.57	\$	49.72	\$	5.10	\$161.50	\$	211.22
18 x 22	0 5	7 5	1	21.67	đ	1 57	4	24.02	đ	F 10	¢ 110 E0	¢	144 52
18 x 32	0.5	7.5	1	21.07	P	1.57	Ą	J4.02	Ą	5.10	\$110.50	P	144.32
	10.15	12.5	1	33.33	\$	1.57	\$	52.33	\$	5.10	\$170.00	\$	222.33
22 x 24	F 10	7 5	1	22.67	¢	1 57	¢	25 50	¢	E 10	¢115.60	¢	151 10
24 x 24	5.10	7.5	1	22.07	P	1.57	Þ	33.39	,	5.10	\$113.00	P	131.19
	0.5	7.5	2	23.00	\$	1.57	\$	36.11	\$	5.10	\$117.30	\$	153.41
	5.10	7.5	2	23.00	\$	1.57	\$	36.11	\$	5.10	\$117.30	\$	153.41
	10.15 15.20	12.5	2	<u> </u>	\$ ¢	1.57	\$ ¢	<u>51.81</u> 67.51	\$ ¢	5.10	\$168.30 \$219.30	\$ \$	220.11
	20.25	22.5	10	53.00	\$	1.57	\$	83.21	\$	5.10	\$270.30	\$	353.51
	25.30	27.5	5	63.00	\$	1.57	\$	98.91	\$	5.10	\$321.30	\$	420.21
24 x 30	10 15	12.5	1	34.00	¢	1 57	¢	53.38	¢	5 10	\$173.40	¢	226.78
	15.20	17.5	1	44.00	\$	1.57	ہ \$	69.08	⇒ \$	5.10	\$224.40	\$	293.48
	20.25	22.5	4	54.00	\$	1.57	\$	84.78	\$	5.10	\$275.40	\$	360.18
24 × 22	25.30	27.5	2	64.00	\$	1.57	\$	100.48	\$	5.10	\$326.40	\$	426.88
24 X 32	10.15	12.5	1	34.33	\$	1.57	\$	53.90	\$	5.10	\$175.10	\$	229.00
24 x 57						-						-	
	20.25	22.5	1	58.50	\$	1.57	\$	91.85	\$	5.10	\$298.35	\$	390.20
12 x 18	25.30	27.5	L	68.50	\$	1.57	\$	107.55	\$	5.10	\$ 349.35	\$	456.90
12 / 10	5.10	7.5	4	20.00	\$	1.57	\$	31.40	\$	5.10	\$102.00	\$	133.40
12 x 24	10.15	12.5	- 10	21.00	-		-	10.67	-	E 10	+ 1 5 0 1 0	+	206 77
	10.15 15.20	12.5	13	31.00	\$ \$	1.5/	\$	48.67	\$	5.10	\$158.10	\$	206.77
	20.25	22.5	6	51.00	\$	1.57	\$	80.07	\$	5.10	\$260.10	\$	340.17
12 x 36													
	10.15	12.5	4	33.00	\$ ¢	1.57	\$	51.81	\$	5.10	\$168.30	\$	220.11
	15.20	17.5	2	43.00	⊅ \$	1.57	⊅ \$	67.51	⊅ \$	5.10	\$219.30	э \$	286.81
12 x 38													
	10.15	12.5	1	33.33	\$	1.57	\$	52.33	\$	5.10	\$170.00	\$	222.33
				Post-T	ensio	oning Be	eam	S			[
18 x 22													
		19	8	44.67	\$	1.57	\$	70.13	\$	5.10	\$227.80	\$	297.93
		30	2	66.67	\$	1.57	\$	104.67	\$	5.10	\$340.00	\$	444.67
		<u>40</u> 50	8 9	106.67	\$	1.57	\$ \$	167.47	\$ \$	5.10	\$442.00	\$	711.47
18 x 25			5	100107	Ŧ	1.07	Ŧ	10/11/	Ŧ	0.10	φοιπου	Ŧ	, 11
22 × 24		50	1	6.67	\$	1.57	\$	10.47	\$	5.10	\$ 34.00	\$	44.47
ZZ X Z4		30	1	67.67	\$	1.57	\$	106.24	\$	5.10	\$345.10	\$	451.34
		40	1	87.67	\$	1.57	\$	137.64	\$	5.10	\$447.10	\$	584.74
22 x 32		10		47.00	¢	1 - 7	¢	72 70	¢	E 10	# 220 70	¢	212.40
		40	2	47.00	\$	1.57	\$ \$	139 73	\$ \$	5.10	\$239.70	\$	593.63
		50	1	109.00	\$	1.57	\$	171.13	÷ \$	5.10	\$555.90	\$	727.03
22 x 36		10		47.67	+	1 5 3	+	74.04	-	E 10	# 2 4 2 4 C	<i>.</i>	217.04
		19	1	47.67	\$	1.57	\$ ¢	74.84	\$	5.10	\$243.10	\$	317.94 598.08
		50	1	109.67	\$	1.57	+ \$	172.18	\$	5.10	\$559.30	\$	731.48
24 x 36	10 (-												
	10.15	12.5	1	35.00	\$ ¢	1.57	\$ ¢	54.95	\$ ¢	5.10	\$1/8.50	\$ ⊄	233.45
	25.30	27.5	2	65.00	₽ \$	1.57	₽ \$	102.05	, , \$	5.10	\$331.50	₽ \$	433.55
					Ċ						Total	\$ 1	4,449.44

Structural Steel:

Size	Lei Ra	ngth Inge	Avg. Length	Quantity	L.F.	Unit Mat'l Cost	Material Cost	Un	it Labor Cost	La	abor Cost	Unit	Equipment Cost	Equipment Cost	Total Cost
C 8 x 12															
W/ 0 ··· 10	10 15	. 15 . 20	12.5 17.5	173 12	2163 210	\$ 10.35 \$ 10.35	\$22,381.88 \$2,173.50	\$ \$	30.50 30.50	\$ \$	65,956.25 6,405.00	\$ \$	3.73 3.73	\$8,066.13 \$783.30	\$ 96,404.25 \$ 9,361.80
W 8X10	10	. 15	12.5	75	937.5	\$ 16.50	\$ 15,468.75	\$	4.06	\$	3,806.25	\$	2.90	\$2,718.75	\$ 21,993.75
W 8 x 21	15	20	17.5	10	175	\$ 10.50	\$ 13,860.00	\$ ¢	4.06	\$	710.50	\$ ¢	2.90	\$2,430.00	\$ 19,700.40
W 10 x 12	15	. 20	17.5	10	1/5	\$ 54.50	\$ 0,057.50	Ŷ	4.00	Ą	/10.50	Ą	2.90	\$ 307.30	\$ 7,233.30
	5.	10	7.5	25	187.5	\$ 19.80	\$ 3,712.50	\$	4.06	\$	761.25	\$	2.90	\$ 543.75	\$ 5,017.50
	10	. 15	12.5	14	175	\$ 19.80	\$ 3,465.00	\$	4.06	\$	710.50	\$	2.90	\$ 507.50	\$ 4,683.00
W 10 v 15	20	. 25	22.5	31	697.5	\$ 19.80	\$13,810.50	\$	4.06	\$	2,831.85	\$	2.90	\$2,022.75	\$ 18,665.10
W 10 X 13	15	. 20	17.5	4	70	\$ 25.00	\$ 1,750.00	\$	4.06	\$	284.20	\$	2.90	\$ 203.00	\$ 2,237.20
W 10 X 19	15	20	17 5	4	70	\$ 36.50	\$ 2 555 00	\$	4 06	\$	284 20	\$	2 90	\$ 203.00	\$ 3,042,20
W 12 x 16	13	. 20	17.5	i	,,,	φ 30.30	<i> </i>	Ψ.	1100	Ψ.	201120	Ψ	2.50	φ 203.00	φ 3/012.20
	10	. 15	12.5	23	287.5	\$ 26.50	\$ 7,618.75	\$	2.77	\$	796.38	\$	1.98	\$ 569.25	\$ 8,984.38
	15	. 20	17.5	14	245	\$ 26.50	\$ 6,492.50	\$	2.77	\$	678.65	\$	1.98	\$ 485.10	\$ 7,656.25
W 12 v 10	25	. 30	27.5	4	110	\$ 26.50	\$ 2,915.00	\$	2.//	\$	304.70	\$	1.98	\$ 217.80	\$ 3,437.50
VV 12 X 19	5.	10	7.5	17	127.5	\$ 36.50	\$ 4.653.75	\$	2.77	\$	353.18	\$	1.98	\$ 252.45	\$ 5,259,38
	15	. 20	17.5	11	192.5	\$ 36.50	\$ 7,026.25	\$	2.77	\$	533.23	\$	1.98	\$ 381.15	\$ 7,940.63
	20	. 25	22.5	24	540	\$ 36.50	\$19,710.00	\$	2.77	\$	1,495.80	\$	1.98	\$1,069.20	\$ 22,275.00
W 14 x 22	15	20	17 5	24	420	+ 42.00	± 10.000.00		2.46		1 022 20	<i>*</i>	1.70	+ 720.20	+ 10 022 40
	15 20	. 20	17.5	24	675	\$ 43.00	\$ 18,060.00	\$	2.46	\$	1,033.20	\$ ¢	1.76	\$ 739.20 \$1.188.00	\$ 19,832.40 \$ 31,873.50
W 14 x 26	20	. 2J	22.5	50	0/5	\$ 5 .00	\$25,025.00	Ą	2.40	Ą	1,000.30	Ą	1.70	\$1,100.00	\$ 51,075.50
	10	. 15	12.5	24	300	\$ 43.00	\$12,900.00	\$	2.46	\$	738.00	\$	1.76	\$ 528.00	\$ 14,166.00
	25	. 30	27.5	10	275	\$ 43.00	\$11,825.00	\$	2.46	\$	676.50	\$	1.76	\$ 484.00	\$ 12,985.50
W 14 x 35	10	45	42.5	-	() F	+ 50.00	+ 2 500 00	-	2.01	-	100.10	-	2.45	+ 124.20	+ 2 022 50
	30	. 15	12.5	5	62.5	\$ 56.00	\$ 3,500.00	\$	3.01	\$	201 20	\$	2.15	\$ 134.38 ¢ 270.50	\$ 3,822.50
W 16 x 26	50		J2.J	4	130	\$ 30.00	\$ 7,200.00	Ą	5.01	Ţ	591.50	Ψ	2.13	\$ 279.30	\$ 7,950.00
	20	. 25	22.5	17	382.5	\$ 43.00	\$ 16,447.50	\$	2.44	\$	933.30	\$	1.74	\$ 665.55	\$ 18,046.35
	25	. 30	27.5	6	165	\$ 43.00	\$ 7,095.00	\$	2.44	\$	402.60	\$	1.74	\$ 287.10	\$ 7,784.70
W 16 x 31	20	25	22.5	26	FOF	t Γ1 00			2 71	<i>+</i>	1 505 25	<i>_</i> +	1.02	±1.100.05	+ 22 540 40
	20	. 25	32.5	26 14	285	\$ 51.00	\$ 29,835.00	\$	2.71	\$ ¢	1,585.35	\$	1.93	\$ 1,129.05	\$ 32,549.40 \$ 25 316 20
W 16 x 40	50	. 55	52.5	14	733	φ 51.00	\$25,205.00	Ψ	2.71	Ψ	1,235.05	Ψ	1.55	\$ 070.15	ψ 23,310.20
	25	. 30	27.5	2	55	\$ 66.00	\$ 3,630.00	\$	3.05	\$	167.75	\$	2.18	\$ 119.90	\$ 3,917.65
W 16 x 57		45		-	07.5	h 00 50								+ +00 =5	
	10	. 15	12.5	7	87.5	\$ 82.50	\$ 7,218.75	\$	3.05	\$	266.88	\$	2.18	\$ 190.75	\$ 7,676.38
W 18 x 13	30	. 55	32.5	5	102.5	\$ 02.50	\$15,400.25	Þ	3.05	Þ	495.05	Þ	2.10	β 354.25	\$ 14,230.13
	10	. 15	12.5	3	37.5	\$ 58.00	\$ 2,175.00	\$	3.67	\$	137.63	\$	1.95	\$ 73.13	\$ 2,385.75
	25	. 30	27.5	1	27.5	\$ 58.00	\$ 1,595.00	\$	3.67	\$	100.93	\$	1.95	\$ 53.63	\$ 1,749.55
W 18 x 35	25	20	27 5	10	220	¢ 50.00	t 10 140 00	<i>t</i>	2 (7	+	1 211 10	+	1.05	¢ 642.50	+ 20.004.CO
	25 30	. 30	27.5	12	195	\$ 58.00	\$ 19,140.00	\$	3.67	\$	715.65	\$ \$	1.95	\$ 380.25	\$ 12,405 90
W 18 x 55	50		52.5	0	175	\$ 50.00	φ11/310.00	Ψ	5.07	Ψ	, 19:09	Ψ	1.75	9 330.2J	÷ 12,403.90
	20	. 25	22.5	3	67.5	\$ 91.00	\$ 6,142.50	\$	3.87	\$	261.23	\$	2.06	\$ 139.05	\$ 6,542.78
	25	. 30	27.5	4	110	\$ 91.00	\$10,010.00	\$	3.87	\$	425.70	\$	2.06	\$ 226.60	\$ 10,662.30
W 20 x 26	30	. 35	32.5	5	162.5	\$ 91.00	\$14,787.50	\$	3.87	\$	628.88	\$	2.06	\$ 334.75	\$ 15,751.13
VV 20 X 20	25	30	27.5	6	165	\$ 72 50	\$ 11 962 50	\$	3 32	\$	547 80	\$	1 76	\$ 290.40	\$ 12 800 70
W 21 x 50	23	. 50	27.5	•	105	ψ /2.50	<i>Q</i> 11,502.50	Ψ	5.52	Ψ	517.00	Ψ	1.70	φ 230.10	φ <u>12</u> ,000.70
	20	. 25	22.5	4	90	\$ 82.50	\$ 7,425.00	\$	3.32	\$	298.80	\$	1.76	\$ 158.40	\$ 7,882.20
	30	. 35	32.5	2	65	\$ 82.50	\$ 5,362.50	\$	3.32	\$	215.80	\$	1.76	\$ 114.40	\$ 5,692.70
W 21 x 68	30	25	32 5	4	30 F	¢ 112.00	¢ 3 640 00	<i>+</i>	2 /1	<i>e</i>	110.02	¢	1 01	¢ E0.00	¢ 3 000 6F
W 24 x 55	50	. 35	52.5	1	52.5	\$112.0U	\$ 3,040.00	4	5.41	\$	110.83	Þ	1.81	φ 30.03	a 2,009.05
	20	. 25	22.5	5	112.5	\$ 91.00	\$ 10,237.50	\$	3.18	\$	357.75	\$	1.69	\$ 190.13	\$ 10,785.38
	25	. 30	27.5	2	55	\$ 91.00	\$ 5,005.00	\$	3.18	\$	174.90	\$	1.69	\$ 92.95	\$ 5,272.85
W 24 x 84	1.5	20	17 -		17.5	+ 120.00	+ 2 (22 F2	-	2.07		F7 00	+		+ 20.45	+ 0 500 40
W 27 × 84	15	. 20	1/.5	1	17.5	\$139.00	\$ 2,432.50	\$	5.27	\$	57.23	Ş	1.74	\$ <i>3</i> 0.45	\$ 2,520.18
W 27 X 04	20	. 25	22.5	10	225	\$139.00	\$ 31,275.00	\$	2.96	\$	666.00	\$	1.58	\$ 355.50	\$ 32,296.50
		-					, ,	Ĺ		ŕ				Total	\$595,649.48

	Size	Length (ft.)	Quantity	L.F.	Unit Mat'l Cost	Material Cost	Unit C	: Labor Cost	Labor Cost	Equ	Unit uipment	Equipment Cost	Total Cost
HSS	6 x 6 x 3/8												
		23	16	368	\$ 880.00	\$14,080.00	\$	49.00	\$ 784.00	\$	35.00	\$ 560.00	\$15,424.00
		15	40	600	\$ 880.00	\$35,200.00	\$	49.00	\$1,960.00	\$	35.00	\$1,400.00	\$38,560.00
		13	24	312	\$ 880.00	\$21,120.00	\$	49.00	\$1,176.00	\$	35.00	\$ 840.00	\$23,136.00
												Total	\$77,120.00

Reinforcing:

Rebar in Concrete											
Location	Rebar	L.F.	Unit Mat'l Cost	Material Cost	Total						
Footing	6 #6	8670	\$ 1.35	\$11,704.50	\$ 11,704.50						
Slab on Grade	#4 @ 12" O.C.	140400	\$ 0.70	\$98,280.00	\$ 98,280.00						
Elevated Slab	#4 @ 24" O.C.	69000	\$ 0.70	\$48,300.00	\$ 48,300.00						
Concrete Column	8 #10	15000	\$ 3.45	\$51,750.00	\$ 51,750.00						
Concrete Beam	6 #7	17000	\$ 1.70	\$28,900.00	\$ 28,900.00						
				Total	\$238,934.50						

Appendix F: General Conditions

Ge	General Conditions Estimate									
Description	Unit	Quantity	Cost/Unit		Total					
Field Personnel										
			\$							
Project Manager	Week	250	1,925.00	\$	481,250.00					
			\$							
Superintendent	Week	250	1,775.00	\$	443,750.00					
			\$							
Asst. Superintendent	Week	250	1,600.00	\$	400,000.00					
			\$							
Asst. Superintendent	Week	250	1,600.00	\$	400,000.00					
			\$							
Field Engineer	Week	250	1,165.00	\$	291,250.00					
			\$							
Asst. Field Engineer	Week	250	895.00	\$	223,750.00					
			\$							
Asst. Field Engineer	Week	250	895.00	\$	223,750.00					
General Expenses			•							
			\$							
Field Trailer 32'x8'	Мо	60	200.00	\$	12,000.00					
			\$							
Office Equipment	Мо	60	155.00	\$	9,300.00					
			\$							
Office Supplies	Мо	60	85.00	\$	5,100.00					
			\$							
Office Telephone	Мо	60	80.00	\$	4,800.00					
			\$							
Office Lights and HVAC	Мо	60	150.00	\$	9,000.00					
			\$		\$					
Temporary Fencing, 6' high	L.F.	30	9.44		283.20					
			\$							
Toilet 1, portable	Мо	60	150.00	\$	36,000.00					
			\$							
Toilet 2, portable	Мо	60	150.00	\$	36,000.00					
			\$							
Toilet 3, portable	Мо	60	150.00	\$	36,000.00					
Permits	Job	1	0.50%	\$	465,000.00					
Final Clean Up	Job	1	0.30%	\$	279,000.00					

Temporary Utilities					
			\$		
Temporary Lighting, 4 floors	CSF/Flr.	581	13.68	\$	31,792.32
			\$		
Temporary Heating, 4 floors	CSF/Flr	581	30.27	\$	70,347.48
			\$		
Temporary Power, 4 floors	CSF/Flr.	581	47.75	\$	110,971.00
Insurance					
Insurance, All-risk type	Job	1	0.25%	\$	232,500.00
Performance Bond	Job	1	0.60%	\$	558,000.00
Scheduling, Large job	Job	1	0.03%	\$	27,900.00
Permits, Rule of thumb	Job	1	0.50%	\$	465,000.00
	•		Sub-Total	\$ -	4,852,744.00
		Loc	cation Factor		0.982
			Total	\$.	4,765,394.61

Appendix G: Detailed Project Schedule



ID 6	Task Name	Duration	Start	Finish	uarter 2nd Quarter	1st Quarter	4th Quarter	3rd Quarter 2nd	d Quarter 1st Quarter 4th Quarter 3rd Quarter
57	Lodge Hang Drywall Walls	77 days?	Tue 12/21/10	Wed 4/6/11	Sep Jan May Sep	Jan May	Sep Jan	May Sep Jan	May Sep Jan May Sep Jan May Sep Jan
58	Lodge Tape & Finish Drowall	76 days?	Mon 12/27/10	Mon 4/11/11					
59	Lodge Flec/Mech Rough-In	73 days?	Wed 1/5/11	Fri 4/15/11					
60	Lodge Hang/Einish Drwall Ceilings	82 days?	Mon 1/17/11	Tue 5/10/11					
61	Lodge Elevator Inspection (S1 & S2)	5 days?	Wed 1/26/11	Tue 2/1/11					
62	Lodge Prime Walls/Ceilings	75 days?	Tue 2/1/11	Mon 5/16/11					
63	Lodge Elevator Installation (P1 & P2)	88 days?	Wed 2/2/11	Fri 6/3/11					
64	Lodge Set Doors/Trims	115 days?	Thu 2/3/11	Wed 7/13/11					
65	Lodge Paint Walls/Ceilings	110 days?	Wed 2/23/11	Tue 7/26/11					
66	Lodge Elec/Plb Fixtures	112 days?	Wed 3/9/11	Thu 8/11/11					
67	Lodge Install Carpets	109 days?	Fri 3/25/11	Wed 8/24/11					
68	Lodge 1st Clean	115 days?	Thu 4/7/11	Wed 9/14/11					
69	Lodge Signage	115 days?	Thu 4/7/11	Wed 9/14/11					
70	Lodge Turner Punchlist	120 days?	Thu 4/14/11	Wed 9/28/11					
71	Lodge Final Walkthrough	120 days?	Thu 4/28/11	Wed 10/12/11	1				
72	Lodge Final Clean	115 days?	Thu 5/12/11	Wed 10/19/11					
73 📑	Lodge Elevator Inspection (P1 & P2)	5 days?	Mon 6/6/11	Fri 6/10/11					1
74 🔳	Lodge Construction Completion	0 days	Wed 10/19/11	Wed 10/19/11					10/19 Lodge Construction Completion
75	Spa Construction	928 days?	Tue 2/19/08	Thu 9/8/11		V			Spa Construction
76	Concrete	122 days?	Tue 2/19/08	Wed 8/6/08		V	Concrete		
77 🔳	Spa Excavate Footings	24 days?	Tue 2/19/08	Fri 3/21/08	5				
78 📑	Spa F/R/P Footings	24 days?	Tue 2/19/08	Fri 3/21/08	5				
79 📑	Spa F/R/P Walls and Footings	23 days?	Wed 2/20/08	Fri 3/21/08	5 E				
80 🔳	Spa Waterproof Walls	72 days?	Wed 4/23/08	Thu 7/31/08	5				
81 🔜	Spa Prep/Pour Slab on Grade	7 days?	Tue 6/24/08	Wed 7/2/08	5	0			
82 🔳	Spa Pour Elevated Slab	10 days?	Fri 7/11/08	Thu 7/24/08	6	0			
83 🔝	Spa Strip/Reshore Elevated Slab	4 days?	Fri 8/1/08	Wed 8/6/08	b				
84	MEP	193 days?	Tue 5/27/08	Thu 2/19/09		φ	WEP		
85 🔳	Spa Underground MEP	18 days?	Tue 5/27/08	Thu 6/19/08	5				
86 📰	Spa MEP Riser Installation	50 days?	Fri 12/12/08	Thu 2/19/09					
87 🔳	Structural Steel	17 days?	Thu 8/7/08	Fri 8/29/08					
88	Roofing	545 days?	Fri 9/5/08	Thu 10/7/10		t	y	- P	Roofing
89	Spa Metal Roof Decking	4 days?	Fri 9/5/08	Wed 9/10/08	5				
90	Spa Light Gauge Roof Trusses	27 days?	Thu 9/25/08	Fn 10/31/08					
91	Spa Roor Sneatning/Deck & Temp Rooring	1 day?	Tue 10/28/08	Tue 10/28/08			1		
92	Spray On Fireprooring	30 days?	Man 2/22/08	FI 3/13/09					
93	Spa Contining Framing/Siteatining	39 days?	Mon 3/23/09	Mod 7/22/09					
95	Spa Ecostar State Roof Installation	20 days?	Mon 8/30/10	Thu 10/7/10					
96	Einishes	167 days?	Mon 10/25/10	Tuo 6/14/11					Einisbes
97 53	Soa Rough-In	24 days?	Mon 10/25/10	Thu 11/25/10					↓ Thisites
98	Spa Hang/Tape/Finish Drywall	15 days?	Tue 11/30/10	Mon 12/20/10					
99	Spa Ceiling Fixtures	20 days?	Tue 12/21/10	Mon 1/17/11					
100	Spa Hardware	29 days?	Mon 1/24/11	Thu 3/3/11					
101	Spa Interior Buildout Complete	0 days	Mon 2/7/11	Mon 2/7/11					2/7 Spa Interior Buildout Complete
102	Spa Casework	34 days?	Mon 2/14/11	Thu 3/31/11					
103	Spa Flooring	28 days?	Mon 3/28/11	Wed 5/4/11					
104 🗔	Spa MEP Trimout	34 days?	Thu 4/28/11	Tue 6/14/11					
105 🗔	Spa 1st Clean	26 days?	Wed 6/15/11	Wed 7/20/11					
106	Spa Turner Punchlist	19 days?	Thu 7/21/11	Tue 8/16/11					
107	Spa Final Clean	18 days?	Tue 8/16/11	Thu 9/8/11					
108 📑	Spa Construction Complete	0 days	Thu 9/8/11	Thu 9/8/11					9/8 🚸 Spa Construction Complete
109	Main Building (MB) Construction	980 days?	Wed 2/27/08	Wed 11/30/11		V	-		Main Building (MB) Construction
110	Concrete	323 days?	Wed 2/27/08	Fri 5/22/09		~		D Concrete	
111 🔳	MB Excavate Footings	25 days?	Wed 2/27/08	Tue 4/1/08	5				
112	MB F/R/P Spread Footings	24 days?	Thu 2/28/08	Tue 4/1/08	5				
Project: Teo Date: Sun 1	h2.1 Task 0/25/09 Solit	Pr M	rogress ilestone	•	Summary Project Summary	Exter	nal Tasks 🛛 🔤 👘 🖾 💭 👘 🖾 🕬 👘 🕬 🖓 🖓	Deadline	Φ
				1.5	rigen cannuty 🗸	÷ Exter	in the state of		
					Pa	age 2			



Appendix H: Proposed Schedule



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ID 6	Task Name	Duration	Start	Finish	2008 2011
1	Salamander Resort and Sna	1306 days?	Thu 3/1/07	Fri 3/2/12	Sep Jan May Sep
2	Preconstruction	493 days?	Thu 3/1/07	Mon 1/19/09	
3	Notice to Proceed	0 days	Thu 3/1/07	Thu 3/1/07	3/1 A Notice to Proceed
4	Design and Development	109 days	Thu 3/1/07	Tue 7/31/07	
5	Building and Health Den Permits	71 days?	Fri 6/22/07	Eri 9/28/07	
6	Construction Documents	67 days?	Thu 7/19/07	Fri 10/19/07	
7	Pricing Documents Complete	0 days	Tue 9/11/07	Tue 9/11/07	9/1 A Pricing Documents Complete
8	GMP Estimate	48 days	Thu 9/13/07	Mon 11/19/07	and a meng becaments complete
0 1	Receive I C Building Permit	40 days:	Wed 9/26/07	Wed 9/26/07	9/26 A Paralyse I C Building Parmit
10	Addendum 2.18	155 days	Eri 12/14/07	Thu 7/17/08	
11	GMP Estimate Review/Value Engineering	115 days?	Mon 11/19/07	Eri 4/25/08	
12	Revised GMP Submission	92 days?	Thu 12/20/07	Eri 4/25/08	
13	Sitework Start	0 days	Mon 10/22/07	Mon 10/22/07	10/22 A Stauent Stat
14	Bouch Crading	102 days	Thu 11/15/07	Mon 4/7/09	
16	Lodgo Lvd 1 Bookfil Wollo	20 days?	Eri 2/29/09	Thu 4/7/08	
16	Crevity Betsining Wall	20 days?	Mon 11/17/08	Mon 1/19/00	-
17	Deminion Dewer Incoming Source	40 udys :	Tue 12/0/08	Tue 12/0/09	120 A Depinion Bound Inserting Sources
18 13	Gas Incoming San/ce	0 days	Thu 1/8/09	Thu 1/8/09	
10	Gas incoming Service	00C days	Mad 4/22/09	Med 44 (46/44	175 🖗 Gas inculting dervice
20	Concrete	136 days?	Wed 1/23/08	Wed 7/30/09	
20	Lodge Excevate Footings	21 days?	Wed 1/23/08	Wed 2/20/08	CONTRACT
22	Lodge E/R/R Spread Eachings	21 days?	Mon 1/28/09	Eri 2/22/00	
22	Lodge F/R/P Spread Foolings	25 days?	Mon 2/19/09	Eri 3/21/09	
20	Lodge Waterproof Walls	23 days?	Mon 2/24/08	Med 4/22/09	
24	Lodge Ground Slab on Grade	23 days?	Mon 3/24/08	Wed 4/23/00	
20	Lodge Globing Slab	16 days?	Word 4/16/08	Wed 4/30/00	
20	Lodge Lvl 1 Point Stab	To days?	Men 4/10/08	Map 4/29/09	
28	Lodge Lvi 7 Four Slab	18 days	Word 4/2 1/08	Fri 5/23/08	
20	Lodge Lvi 2 Pour Slab/Columns	21 days?	Mon 5/5/08	Mon 6/2/08	
30	Lodge Lvl 2 Four Glab/Courtins	43 days:	Tue 5/6/08	Thu 7/3/08	
31	Lodge Lvi 3 Earm Slab	40 days?	Med 5/14/08	Eri 6/6/08	
22	Lodge Lvi 3 Strip/Reshere/Cure/Remove	22 days?	Thu 5/15/09	Eri 6/12/09	
32	Lodge Lvl 2 Strip/Reshore/Cale/Remove	15 days?	Eri 5/23/08	Thu 6/12/08	
34	Lodge Penthouse Strip/Resbore/Cure/Remove	18 days?	Mon 5/26/08	Wed 7/30/08	
35	Lodge Penthouse Ly Form Slab	18 days:	Wed 5/28/08	Eri 6/20/08	
36	Lodge Penthouse Lvi F/R/D Elevated Slab	14 days:	Eri 6/6/08	Med 6/25/08	
37	MEP	130 days?	Mop 3/17/08	Fri 9/12/08	MEP
38 =	Lodge Underground MEP	32 days7	Mon 3/17/08	Tue 4/29/08	· · · · · · · · · · · · · · · · · · ·
39	Lodge Underground MEL	18 days?	Eri 4/18/08	Tue 5/13/08	
40	Lodge Lvl 2 In Slab MEP	20 days?	Eri 5/2/08	Thu 5/29/08	
41	Lodge Lvl 3 in slab MEP	18 days?	Fri 5/16/08	Tue 6/10/08	
42	Lodge Penthouse Lvl In Slab MEP	14 days?	Wed 6/4/08	Mon 6/23/08	
43	Lodge MEP Riser Installation	50 days?	Mon 7/7/08	Fri 9/12/08	
44	Roofing	293 days?	Mon 8/11/08	Wed 9/23/09	C Roofing
45	Lodge Light Gauge Roof Trusses	59 days?	Mon 8/11/08	Thu 10/30/08	· · · · · · · · · · · · · · · · · · ·
46	Lodge Roof Sheathing & Temp Roofing	35 days?	Mon 10/13/08	Fri 11/28/08	
47	Lodge Roof Installation	155 days?	Mon 12/1/08	Fri 7/3/09	- <u>-</u>
48	Lodge Chimney Framing	63 days?	Fri 1/2/09	Tue 3/31/09	
49	Lodge Chimney Sheathing	39 days?	Thu 2/5/09	Tue 3/31/09	
50	Lodge Expansion Joints	55 days?	Mon 5/18/09	Fri 7/31/09	
51	Lodge Chimney Stucco	27 days?	Mon 6/22/09	Tue 7/28/09	
52	Lodge Ecostar Slate Roof Installation	41 days	Wed 7/29/09	Wed 9/23/09	
53	Lodge Initial Dry In	0 davs	Mon 7/20/09	Mon 7/20/09	7/20 🔷 Lodge Initial Dry in
54	Lodge Core & Shell Complete	0 davs	Thu 9/24/09	Thu 9/24/09	9/24 & Lodge Core & Shell Complete
55	Finishes	234 days?	Fri 10/29/10	Wed 9/21/11	Finishes
56	Lodge Elevator Installation (S1 & S2)	88 davs	Fri 10/29/10	Tue 3/1/11	
Project: Se	chedule Analysis.mpp Task	F	Progress	6	Summary V External Tasks Deadline 🕀
Date: Sat	3/27/10 Split	N	Ailestone	\diamond	Project Summary V External Milestone 🧄
-	1				Des 4
					Page 1

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Appendix I: Proposed General Conditions

	General Conditions Savings									
Description	Unit	Quantity	Cost/Unit		Total					
Field Personnel										
Project Manager	Week	43	\$ 1,925.00	\$	82,775.00					
Superintendent	Week	43	\$ 1,775.00	\$	76,325.00					
Asst. Superintendent	Week	43	\$ 1,600.00	\$	68,800.00					
Asst. Superintendent	Week	43	\$ 1,600.00	\$	68,800.00					
Field Engineer	Week	43	\$ 1,165.00	\$	50,095.00					
Asst. Field Engineer	Week	43	\$ 895.00	\$	38,485.00					
Asst. Field Engineer	Week	43	\$ 895.00	\$	38,485.00					
General Expenses										
Field Trailer 32'x8'	Мо	10	\$ 200.00	\$	2,000.00					
Office Equipment	Мо	10	\$ 155.00	\$	1,550.00					
Office Supplies	Мо	10	\$ 85.00	\$	850.00					
Office Telephone	Мо	10	\$ 80.00	\$	800.00					
Office Lights and HVAC	Мо	10	\$ 150.00	\$	1,500.00					
Temporary Fencing, 6' high	L.F.	30	\$ 9.44	Ş	283.20					
Toilet 1, portable	Мо	10	\$ 150.00	\$	6,000.00					
Toilet 2, portable	Мо	10	\$ 150.00	\$	6,000.00					
Toilet 3, portable	Мо	10	\$ 150.00	\$	6,000.00					
Permits	Job	1	0.50%	\$	465,000.00					
Final Clean Up	Job	1	0.30%	\$	279,000.00					
Temporary Utilities		· · · · · · · · · · · · · · · · · · ·								
Temporary Lighting	CSF/Flr.	581	\$ 13.68	\$	6,358.46					
Temporary Heating	CSF/Flr	581	\$ 30.27	\$	15,828.18					
Temporary Power	CSF/Flr.	581	\$ 47.75	\$	24,968.48					
Insurance	1									
Insurance, All-risk type	Job	1	0.25%	\$	232,500.00					
Performance Bond	Job	1	0.60%	Ş	558,000.00					
Scheduling, Large job	Job	1	0.03%	\$	27,900.00					
Permits, Rule of thumb	Job	1	0.50%	Ş	465,000.00					
			Sub-Total	\$	495,620.12					
		Lo	cation Factor		0.982					
			Total Savings	Ś	486.698.96					

Appendix J: Proposed Security Cost

General Conditions Estimate									
Description	Unit	Quantity	Cost/Unit		Total				
Temporary Security									
			\$						
Watchman	Hr	2580	25.00	\$	64,500.00				
			\$						
Watchman, Overtime	Hr	4644	37.50	\$	174,150.00				
			Sub-Total	\$	238,650.00				
		Location Factor			0.982				
			Total Cost	\$	234,354.30				

Appendix K: Shrub Replacement Plan

Propo	sed	Native Rep	acement
Fothergilla		Button Bush	
Cherry Laurel		Virginia Sweetspire	
Otto Luyken Cherry Laurel		Henry Garnet's Sweetspire	
San Jose Holly		Red Chokeberry	

	Final Report 2010
Korean Spice Viburnum	Arrowwood Viburnum
Shasta Doublefile Viburnum	Possumhaw Viburnum
Nandina	Winterberry
Vernal Witchhazel	Common Witchhazel

Appendix L: Irrigation System Specifications

Below Ground Cistern Tank Collection System



(Note how overflow is directed away from and downhill from the tank excavation and housing structure and must drain at the surface). (Follow underground cistern installation instructions

carefully which are included with each tank.)

The Complete System package detailing a 1200 gallon underground cistern (PN10010--).

- A. Leaf Eater (RCLE3).
- B. First Flush In-Ground Diverter (RCID12).
- C. 4" Overflow (RCOF4).
- D. 12" Riser.

E. Submersible Pump (RCSUBPUMP) with controls (RCCONTROL).

- F. Filter Pit (optional) (RCFP).
- G. 3 Cord Seal (RC3CS).

Snyder Industries has the solution for your rain harvesting needs. Snyder is a leading manufacturer of exceptionally performing polyethylene water tanks and offers a full line of rain harvesting systems and components.

Rainfall in	Square Feet of Roof Surface											
Inches	1,000	2,000	3,000	4,000	5,000	10,000						
		Gallons of Water										
1	625	1,250	1,875	2,500	3,125	6,250						
2	1,250	2,500	3,750	5,000	6,250	12,500						
5	3,125	6,250	9,375	12,500	15,625	31,500						
10	6,250	12,500	18,750	25,000	31,250	62,500						
30	18,750	37,500	56,250	75,000	93,750	187,500						
40	25,000	50,000	75,000	100,000	125,000	250,000						
50	31,250	62,500	93,750	125,000	156,250	312,500						
60	37,500	75,000	112,500	150,000	187,500	375,000						
			Gallons of Wa	ater Captured								

Potential Annual Rainwater Collection

Calculation: annual rainfall per inch x 625 gallons per 1,000 square feet roof surface.

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Appendix M: Rainwater Collection Areas

