## The Salamander Resort and Spa Middleburg, Virginia

## Final Report

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## The Osalamander Resort \& Ospa

## General Building Data

Location: Middleburg, Virginia Occupancy: Mixed use, Hotel, Spa
Size: 230,000 sf
Height 4 plus mechanical penthouse Construction Cost: $\$ 93$ million
Construction Dates: Spring 2008 - Spring 2011


- Basement $5^{n}$ slab on grade with typical $18 \times 18$ and $24 \times 24$ concrete columns
- Converts from concrete to steel columns on 1st floor main building
- Guest house $9^{\prime \prime}$ or $10^{\prime \prime} 3500$ psi post tensioned concrete slab
- Lightgage steel roof trusses at $48^{*}$ on center
- Spread footings minimum $36^{\prime \prime}$ below grade
- 1950 gpm cooling tower on main roof serves chillers 1-3 - 15 AHU's. (9) Variable Frequency Drive. (6) Constant Volume - (6) additional heat recovery AHU's

Lighting / Electrical

- 3200A 480/277V 3 phase, 4W main switchboard - Uninterrupted Power Supply (UPS) for 4th floor lodge - Indoor emergency diesel generator, $650 \mathrm{~kW} 480 / 277 \mathrm{~V}$
- Secondary $120 / 208 \mathrm{~V} 3$ phase
- Dimmable neon 2400 K "incandescent" tubing used in spa coves


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## Executive Summary

The purpose of this senior thesis is to study The Salamander Resort and Spa, which is located in Middleburg, Va. This report contains a project overview and three analyses focusing on schedule deceleration, guest lodge lighting redesign, and water management. The analyses are focused on reducing the upfront and running costs of the resort through the use of lower energy use and alternate scheduling.

The first analysis deals with the voluntary schedule deceleration per owner's request. The initial design and schedule called for completion in March 2011, but was delayed 12 months to March 2012. In the revised schedule, most activities were not delayed, rather their durations were extended over a longer period of time. The main exception to this schedule was the interior work. From January 2009 to November 2009, all interior work in the lodge was stopped. I analyzed a halt in construction activities for a period of ten months. This will alleviate the general conditions costs for that time period while still allowing the project to finish by March 2012. The general conditions savings totaled $\$ 252,345$. The main component of the savings came from the project team salaries and temporary power, lighting, and heating.

The second analysis deals with the redesign of the guest lodge lighting system. A large amount of energy is wasted every year when occupants leave lights on when they are not in the room. The resort has 168 rooms and this leads to a significant energy waste. I analyzed a system that will replace all halogen lamps with LED's and install a control system that will turn off the guest room's lights when no one is present. The total energy cost per year with the LED's is $\$ 5,151$ versus $\$ 60,584$ with halogens. Taking initial investment, replacement cost, and yearly energy cost into consideration, the payback period for the proposed system is 2.37 years. Approximately $\$ 100,000$ will be saved in energy and maintenance costs annually for the following 15 years.

The final analysis investigates the buildings water management, more specifically, the irrigation system. A wide range of plants are used in the surrounding landscaping, many of which are not native to Virginia. Native plants are accustomed to the climate and conditions of the location and are hardier and more likely to survive harsh conditions than that of non-native and exotic plants. By replacing the current pond pump irrigation water source with rain water collection tanks, the system improves sustainability. The additional cost of the proposed system is $\$ 18,350$.

## Project Overview

## Introduction

Building Name: Salamander Resort and Spa
Location and Site: Middleburg, Virginia. 340 acres
Building Occupant Name: Salamander Hospitality
Occupancy: Mixed use. Hotel, spa, equestrian center
Size (total square feet): $230,000 \mathrm{ft}^{2}$
Building Cost: $\$ 93$ million
Dates of Construction: March 2007 - March 2011

## Building Enclosure:

Building Facades: There are two major exterior wall facades on the Salamander Resort and Spa. Stone and stone veneer is used on the main entrance building, front and rear. The stone is used on the lower portion of the wall and the stone veneer is used on the middle and upper portion to reduce overall weight. The remainder of the main building and guest wing is stucco.


Roofing:
The roof consists of three different types. Composite slate shingle roofing is used on all slanted roofs. EPDM single ply - fully adhered (TPO) or Modified Bituminous Irma Roofing Systems are used for all flat roofs, usually found in the mechanical spaces.

## Client Information

Salamander Hospitality is a company formed in 2005 in part by the current CEO Sheila Johnson. Her goal is to grow the company by acquiring one of a kind properties and managing them to provide an unforgettable experience. Salamander Hospitality specializes in the management of luxury resorts and hotels, like the Salamander Resort and Spa. Their focus on owner and customer satisfaction is what drives this company. Salamander Hospitality also owns
and manages the Innisbrook Resort and Golf Club in Florida and the Woodlands Inn in South Carolina. Both of these properties reflect the mission that Salamander Hospitality set out to achieve.

## Owner Expectations

Cost: Most of the funding for this project comes directly from Salamander Hospitality. The high-end nature of this project can lead to changes in interior and exterior finishes throughout the project and it is one of the goals of the contractor to minimize the cost impact of these.

Quality: The owner is looking for a very high quality finished product that will serve the needs of her client base. Only the finest fixtures, furniture, and wood details are used. This requires the contractor to provide special attention to the installation process in order to ensure the best looking product.

Schedule: The owner had initially set a Spring 2010 completion date but has since pushed that back to Spring 2011. This has provided the contractor with significant float time, and allows for easier trade coordination.

Safety: It is critical that the contractor provide a safe environment for all the workers on site.

## Local Conditions



Figure (1)


Figure (2)

The project is located in the town of Middleburg, Virginia on route 50 , about 40 miles west of Washington D.C. and 120 miles north of Richmond (Figure 1). Figure (2) shows the site boundary in blue and the resort location in red.

Preferred Methods of Construction: Much of the residential properties in old town Middleburg are masonry and brick construction. To match this look, Salamander Resort uses a stone façade on the main entrance area.

Construction Recycling: All recycling is collected on site and removed by a third party company to a local recycling plant.

Tipping Fee: In 2008, the tipping fee in Loudon County is $\$ 60 /$ ton. (Loundon County Solid Waste Management Planning District)

Soil Type: The regional soil consists of a blend of deep, well drained, silty soils and clays. During footing excavation, the subsurface water level was not reached.

## Project Delivery System

The project delivery method used is a design build. A design build method was chosen because a large portion of the lighting system and custom interior work was not designed at bid time. When Turner took over the project in 2007 it had already undergone three complete redesigns under a different general contractor. The owner initially wanted to fast-track the process to make up for lost ground but this was later altered to fit their needs more accurately.

The Owner, Salamander Hospitality, holds direct contracts with all the design architects, engineers, consultants. The contract between the Owner and Turner Construction is a Cost Plus Fee with a Guaranteed Maximum Price (GMP). Turner Construction holds lump sum contracts with all the subcontractors. The contractor was selected through a competitive process based upon qualification, fee, and a general conditions proposal.

Turner Construction uses a Contractor Controlled Insurance Program (CCIP) which includes workers compensation and general liability. The Owner separately purchased Builders Risk Insurance. There is no Performance Bond on this project.

See the following page for the project delivery system organizational chart.


## Staffing Plan

Turner Construction has eight people working on site, one project manager, three superintendents, and four engineers. There are more people on this site than usual because of the complexity of the systems and installation. The Project Engineer and Assistant Engineer are responsible for RFI's and submittals on a daily basis. The Superintendents are responsible for work flow, schedule changes, and subcontractors. Below is the Turner Construction on-site staff.


## Building Overview and Systems Summary

| Building Systems Summary |  |  |  |
| :---: | :---: | :--- | :--- |
| Yes | No | Work Scope |  |
|  | X | Demolition required? |  |
| $X$ |  | Structural steel frame | Mobile crane for erection |
| $X$ |  | Cast in place concrete | Crane and bucket placement. Wood formwork |
|  | $X$ | Precast concrete |  |
| $X$ |  | Mechanical system | Mechanical room located in basement of main <br> building, northeast corner. Dry sprinkler system |
| $X$ |  | Electrical system | Main 3200A 480/277 - 3 phase 4W and <br> secondary120/208V - 3 phase 4W |
| $X$ |  | Masonry | Stone veneer on main building at entrance |
|  | X | Curtain wall |  |
|  | $X$ | Support of excavation |  |

Excavation:

- All foundations should be a minimum of 36 " below grade
- Building spread and strip footings shall bear on undisturbed natural soils or compacted fill with a bearing pressure of 3500 psf .
- Utility lines shall not be placed through of below foundations without structural engineer's approval

Concrete:

- A 3000 psi reinforced concrete was used for 5 " interior slab on grade
- The guest house utilized 9 " and 10 " 3500 psi post tensioned reinforced concrete on metal deck with continuous welded wire fabric.
- Typical $16 \times 28$ reinforced concrete columns utilized in guest house.

Structural Steel:

- Rolled shapes and Round HSS Shapes - ASTM A992, ASTM A500
- 2" 18 gage Lok-Floor composite metal decking used in the guest house
- $1 \frac{1}{2 \prime \prime}$ deep, wide rib, 20 gage galvanized roof decking used for both the guest house and main building
- Lightgage steel roof trusses with 8 " lightgage purlin at 48 " on center

Mechanical System:

- 15 main AHU's, 9 Variable Frequency Drive (VFD) and 6 Constant Volume (CV)
- 6 heat recovery AHU's. 3 located in the main lodge, 2 in the spa, and 1 in the laundry room
- 1950 gpm cooling tower located on the main roof serves chillers 1-3
- Mechanical room located in basement in north east corner

Electrical System:

- From utility, main 3200A 480/277V - 3 phase 4W switchboard with secondary 120/208V 3 phase
- Uninterrupted Power Supply (UPS) for $4^{\text {th }}$ floor guest house and $1^{\text {st }}$ floor main building
- Indoor emergency diesel generator ( $650 \mathrm{~kW} 480 / 277 \mathrm{~V}-3$ phase 4 W )
- Custom designed light fixtures and chandeliers


## LEED Design Features:

- "Green" slate roofing made from recycled rubber and PVC piping
- Minimize irrigation by using native plants and species
- Maximize opportunity to use building materials made from recycled products
- Use low emitting paints, carpets, and window treatments
- Protection procedures in place to conserve 250 of the 340 acres


## Site Plan of Existing Conditions

See Appendix A for Site Plan of Existing Conditions

## Site Layout Planning

See Appendix B for Site Layout Planning
The most critical phase of this project is the finishing phase. Due to the large number of custom designed fixtures and materials, transportation and handling should be kept to a minimum to avoid damage. Material storage locations are placed by the guest lodge and restaurant on the northern side of the site, to minimize movement. Turner Construction does not supply any material hoists so the subcontractors should place their hoist in the center section of the guest lodge. The main building and spa is one floor so a hoist is not needed to access those areas.

The on-site trailer and temporary parking is located in the designed parking lot for guests. Dumpsters and recycling collectors are located to the east of the main building, which has relatively flat graded land for easy pick-up. Entrance and exit will on the existing 2-way paved
road. Due to the large area and remoteness of the site, no barrier fences are used except for chain link gates at the two entrance and exits.

The planning and coordination for this project benefits greatly from being located on a relatively flat piece of land in the middle of a large open field, free of trees, surrounding buildings, and vehicular/pedestrian traffic. It allows for a lot of freedom when designing a site layout.

## Project Cost Evaluation

| Actual Costs |  |  |
| :--- | :--- | ---: |
| Construction Cost | $\$$ | $93,802,046.00$ |
| Construction Cost/SF | $\$$ | 409.24 |
| Total Project Cost Estimate | $\$$ | $135,280,000.00$ |
| Total Project Cost Estimate/SF | $\$$ | 590.19 |


| Building Systems Costs (Cost and Cost/SF) |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: |
| Building System | Cost |  | Cost/SF |  |
| Excavation and Fill | $\$$ | 277,443 | $\$$ | 1.21 |
| Building Concrete | $\$$ | $7,191,105$ | $\$$ | 31.37 |
| Structural Steel and Metal Deck | $\$$ | $2,023,292$ | $\$$ | 8.83 |
| Plumbing | $\$$ | $13,766,120$ | $\$$ | 60.06 |
| Electric | $\$$ | $10,674,385$ | $\$$ | 46.57 |
| Finish Carpentry \& Architectural Millwork | $\$$ | $4,120,000$ | $\$$ | 17.98 |
| Windows, Doors, and Glazing | $\$$ | $1,285,355$ | $\$$ | 5.61 |
| Gypsum Drywall Work | $\$$ | $5,911,608$ | $\$$ | 25.79 |
| General Requirements | $\$$ | $1,893,275$ | $\$$ | 8.26 |

The Total Project Cost Estimate includes land, design/consultant fees, furniture, fixtures, equipment, and development/marketing in addition to construction costs. The Turner Construction Project Manager on site estimated all these additional costs because the owner did not release the official data. As seen in the chart, the most expensive systems in the building are the plumbing and electric.

Three different project cost analysis were performed throughout the course of the fall semester. The most basic estimate was performed using the software D4Cost. The next estimate was done using R.S. Means Square Foot and the final estimate was a detailed structural systems estimate. The three estimates can be seen below.

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## D4Cost Estimating

See Appendix C for the detailed D4Cost Estimate

The D4Cost estimate was calculated by selecting two similar projects in the database and combining their attributes into one estimate. The closest projects that D4 had in the database were motel/hotels. I chose The Hampton Inn and Suites Hotel because it is a high end hotel located in Chicago. The Inn on Lake Superior is more similar because of the amenities offered but does not come close to the luxury that the Salamander Resort offers.

| Projects Used in D4 Cost Estimate |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Use | Project Name | Size (SF) | Floors | Building Cost |  |
| Hotel/Motel | Hampton Inn and Suites Hotel | 162,000 | 12 | $\$ 13,797,591$ |  |
| Hotel/Motel | The Inn on Lake Superior | 65,345 | 3 | $\$ 4,073,012$ |  |


| Parametric D4Cost Estimate |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Division | Name | Percent | Sq. Cost |  | mount |
| 0 | Bidding Requirements | 7.22 | \$ 9.60 | \$ | 2,200,317 |
| 1 | General Requirements | 3.75 | \$ 4.99 | \$ | 1,144,345 |
| 2 | Site Work | 4.08 | \$ 5.42 | \$ | 1,242,343 |
| 3 | Concrete | 21.00 | \$ 27.92 | \$ | 6,400,073 |
| 4 | Masonry | 6.09 | \$ 8.09 | \$ | 1,855,012 |
| 5 | Metals | 1.56 | \$ 2.07 | \$ | 475,578 |
| 6 | Wood \& Plastics | 4.78 | \$ 6.36 | \$ | 1,457,405 |
| 7 | Thermal \& Moisture Protection | 1.91 | \$ 2.54 | \$ | 581,897 |
| 8 | Doors \& Windows | 6.39 | \$ 8.50 | \$ | 1,947,182 |
| 9 | Finishes | 11.38 | \$ 15.13 | \$ | 3,467,559 |
| 10 | Specialties | 0.35 | \$ 0.47 | \$ | 107,688 |
| 11 | Equipment | 0.09 | \$ 0.12 | \$ | 26,443 |
| 12 | Furnishings | 0.05 | \$ 0.06 | \$ | 13,973 |
| 13 | Special Construction | 0.68 | \$ 0.90 | \$ | 206,514 |
| 14 | Conveying Systems | 2.49 | \$ 3.31 | \$ | 758,084 |
| 15 | Mechanical | 19.61 | \$ 26.08 | \$ | 5,978,605 |
| 16 | Electrical | 8.60 | \$ 11.43 | \$ | 2,620,616 |
|  | Total Building Costs | 100.00 | \$ 132.99 | \$ | 30,483,633 |

The D4Cost estimate reported at $\$ 132.99 / \mathrm{SF}$ with a total project cost of $\$ 30,483,633$. This value is approximately one third of the actual project cost. This significant difference is due in part to the use of the building. The D4 projects are mainly hotel oriented while the Salamander project is a full resort with spa, restaurant, guest rooms, and horse stables/pastures. The Hampton Inn utilizes precast concrete, whereas The Salamander Resort does not. If the three projects had more similar structural, mechanical, and electrical systems the estimate would be closer. The amount of custom interior work, lighting fixtures and woodwork, found on this project also contributes to the difference.

## R.S. Means Square Foot Estimate

See Appendix D for the reference pages from R.S. Means 2009

The following R.S. Means square foot estimate is based off M.350: 4-7 Story Hotel with Face Brick and Concrete Back-Up. The costs are calculated using an area of 229,213 square feet and $2,828^{\prime}$ perimeter. Basement addition along with height, perimeter, and location adjustments were used. A majority of the structural framing is done with reinforced concrete.

| Exterior Wall | S.F. Area | 195,000 |
| :--- | :--- | ---: |
|  | L.F. Area | 850 |
| Face Brick with <br> Concrete Block Back-up | Steel Frame | $\$ 159.60$ |
|  | R/Conc. Frame | $\$ 157.60$ |

Story Height Adjustment:

$$
12^{\prime}-10^{\prime} 3^{\prime \prime}=1.75^{\prime}
$$

$$
-\$ 1.25 / \mathrm{ft} *(1.75)=-\$ 2.19 / \mathrm{sq} . \mathrm{ft} .
$$

Perimeter Adjustment:

$$
2828^{\prime}-850^{\prime}=1978^{\prime}
$$

$$
+\$ 1.75 / 100 \mathrm{ft} *\left(1978^{\prime}\right)=+\$ 34.62
$$

Basement Addition:
$+\$ 32.20 / \mathrm{sq} \mathrm{ft}$

Sub-Total Per Square Foot Estimate:
$157.60+2.19+34.62+32.20=\$ 226.61 / \mathrm{sq} \mathrm{ft}$.

Project Location Adjustment
Arlington, Virginia is the closest location listed in RS Means.
$\$ 226.61 * 0.93=\$ 210.75 / \mathrm{sq} \mathrm{ft}$.

Sub-Total Construction Cost

$$
\$ 210.75 / \mathrm{sq} \mathrm{ft.} *(229,213 \mathrm{sq} \mathrm{ft.})=\$ 48,306,640
$$

Common Additives:
(5) 5000 lb . capacity elevators @ \$170,700 each $\rightarrow+\$ 853,500$
(1) Security camera and monitor @ $\$ 1850$ and
(37) additional cameras @ \$1000 each $\rightarrow+\$ 38,850$
(4) 125 lb . washers @ $\$ 32,800$ each $\longrightarrow+\$ 131,200$
(1) 50 lb . washer @ \$12,200 each $\rightarrow+\$ 12,200$
(2) Laundry folders @ \$66,500 each $\rightarrow+\$ 133,000$
(1) Laundry ironer @ \$35,500 each $\longrightarrow+\$ 35,500$

Common Additives Total $=+\$ 1,204,250$

Total Construction Cost:

$$
\$ 48,306,640+\$ 1,204,250=\$ 49,510,890
$$

$$
\$ 216.00 / \mathrm{SF}
$$

The R.S. Means square foot estimate is about $\$ 45$ million less than the actual project cost. Part of this difference can be attributed to using a hotel as the basis for the estimate, as R.S. Means does not have a category for resorts. Another reason for the difference comes in the façade, R.S. Means uses Face Brick with Concrete Clock Back-Up while The Salamander Resort uses a stone veneer and stucco. Other discrepancies were discussed in the analysis of the D4Cost estimate.

The R.S. Means estimate for this project is more accurate than the D4Cost estimate partly because the Means estimate is more tailored to this project, while the D4 Cost is based off other buildings. While neither estimate had a good basis for comparison, the R.S. Means estimate is would be fairly accurate if it included more of the specialty items and finishes found in The Salamander Resort.

## Detailed Structural Systems Estimate

See Appendix E for detailed structural systems estimate


Assumptions:

- Location Factor, Arlington $=.982$
- 2 use plywood was used for forming
- No waste factors were used
- Footings used 6 \#6 for reinforcing
- Slab on grade used \#4 @ 12" O.C. for reinforcing
- Concrete Beams used 6 \#7 for reinforcing
- Concrete Column used 8 \#10 for reinforcing
- Elevated slab used \#4 @ 24" O.C. for reinforcing
- Concrete CY totals do not exclude volume of rebar

The detailed structural estimate was performed using R.S. Means 2009. Due to the irregularity of my project I was unable to do a simple estimate of a typical bay and extrapolate. The guest lodge is the only area that has a repeatable structural system. In order to simplify the take-off of concrete beams, concrete columns, and steel members, I used a length range method. I grouped all the different sized beams and columns into length ranges. For example, I counted up all 24 "x $24^{\prime \prime}$ concrete beams and categorized them as either $10^{\prime}-15^{\prime}, 15^{\prime}-20^{\prime}, 20^{\prime}-25^{\prime}$, etc. I then took the average length, in this case $12.5^{\prime}, 17.5^{\prime}, 22.5^{\prime}$, and multiplied it by the quantity and size to get cubic yards of concrete. I used a similar method for the concrete columns, footings, and steel members. As seen in the above assumptions, I used uniform reinforcing for slabs, beams, and columns to simplify to the take-off. The total structural cost for the project is $\$ 1,337,016.57$.

The actual cost of building concrete from the GMP estimate by Turner Construction is $\$ 7,191,105$. This number is significantly larger than the value that I obtained for structural concrete. The main reason for this difference is that this work was performed by a subcontractor
who also had to excavate and backfill the footings. The actual estimate also includes concrete used for paving, sidewalks, and retaining walls. I also did not take into account the additional material and labor costs of post tensioned concrete in the guest lodge.

## General Conditions Estimate

See Appendix F for General Conditions Estimate
Assumptions:

- Location factor, Arlington $=.982$
- Turner Construction employees are on site for entire duration of project
- Project duration: 5 years $=60$ months

| General Conditions Summary |  |  |
| :--- | :--- | :---: |
| Item | Cost | $\%$ of GC |
| Field Personnel | $\$ 2,419,402.50$ | $50.8 \%$ |
| General Expenses | $\$ 876,418.50$ | $18.4 \%$ |
| Temporary Utilities | $\$$ | $209,274.81$ |
| Insurance | $\$ 1,260,298.80$ | $26.4 \%$ |
| Total | $\$ 4,765,394.61$ | $100.0 \%$ |

The General Conditions estimate was performed using R.S. Means 2009. The estimate was broken up into four categories, field personnel, general expenses, temporary utilities, and insurance. The estimate came to $\$ 4,765,394.61$ which is $5.1 \%$ of the total construction cost. The largest portion of the cost estimate, roughly $50 \%$, is from field personnel because Turner Construction has seven employees on-site.

## Detailed Project Schedule

See Appendix G for a detailed Project Schedule

The Salamander Resort and Spa schedule is broken up into the construction of three buildings, the guest lodge, the spa, and the main building.


The three buildings each begin and finish construction at about the same time. The important dates are shown below.

| Building | Start | Finish | Duration <br> (days) |
| :--- | :---: | :---: | :---: |
| Lodge | $1 / 23 / 2008$ | $12 / 19 / 2011$ | 976 |
| Spa | $2 / 19 / 2008$ | $9 / 8 / 2011$ | 928 |
| Main Building | $2 / 27 / 2008$ | $11 / 30 / 2011$ | 980 |

Construction of the spa and main building are very similar except for the inclusion of more structural steel in the main building. Project Substantial Completion occurs less than a week after the finish of the main building. Closeout takes roughly two months, and the building is handed over to the owner in March 2012. In 2008 the schedule was modified and delayed by a year to accommodate the owner's wishes. This change can be seen very clearly in the delay between the structure of the buildings and the finishes. Between January and November 2009, all interior work was put on hold. After the schedule adjustment, the total duration of the project is exactly five years, March 1, 2007 to March 2, 2012

## Foundation

The total duration of the foundation work was approximately 6 months. The foundation consisted of reinforced concrete spread footings excavated down to a minimum of 36 inches below the slab on grade. No formwork was needed because the excavation holes were dug to the correct footing size.

## Structural

Superstructure for The Salamander Resort and Spa took about 6 months to complete. It consisted of both concrete and steel framing. The basement and guest wing have concrete

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framing throughout and the main building has steel framing on the first floor. This created challenges in the schedule when both concrete and steel were being installed simultaneously.

## Finishes

Turner has allocated a large amount of time to the finishes due to the complexities of the project. A majority of the fixtures are custom designed for this project and are more likely to require additional time to install. Like many of the activities, finishes would be able to finish in less time than the allotted if the schedule was optimized for time.

## Analysis I: Schedule Deceleration

## Introduction

The initial design and schedule for The Salamander Resort and Spa called for completion in March 2011. Per owner's request, the project was delayed 12 months to March 2012. This was done for a variety of reasons that will be discussed later. This intentional deceleration of the schedule creates more work for the contractor. The cost impact is significant, mainly due to the additional year of general conditions. For this analysis, I am proposing a halt in construction activities for a period of ten months. This will alleviate the general conditions costs for that time period while still allowing the project to finish by March 2012.

## Original Schedule: Completion date March 2011

The Salamander Resort and Spa schedule involves the construction of three interconnected areas, the guest lodge, the spa, and the main building. The breakdown of areas is seen in the schematic below.


The Main Building and Spa are smaller in square footage than the lodge because they are one floor compared to four. Due to the repetitive nature of the guest lodge, the three building areas all finish in roughly the same amount of time, from exteriors to interiors.

The critical path for The Salamander Resort and Spa followed that of a typical commercial construction project. It includes the following activities; concrete frame, steel frame, core/shell, enclosure, and interiors. A delay to any of these activities would cause a delay in the overall project completion date. The roof dry-in milestone was the most critical point in the schedule as it occurred between the enclosure and the interiors. This was an important point to reach
because it then allowed the extensive interior work to begin. The importance of this milestone was later negated due to the one year delay the owner placed on the entire project.

## Revised Schedule: Completion date March 2012

See Appendix G for Revised Schedule

The revised schedule in Appendix $G$ is the schedule created immediately following the decision to extend the project by 12 months. Therefore, it shows a gap in activities similar to that of my proposed schedule in Appendix H. What differs between this revised schedule and the actual work is that the activities preceding the break were not on schedule and ran into the extension period.

The revised schedule has a project completion date of March 2012. The owners of The Salamander Resort and Spa decided it was in their best interest to delay the project by one full year. One of the main reasons for this decision was the current economic climate. They felt that if the resort opened up in March 2011, and the economy had not recovered, they could not rent out all the rooms to capacity. The owner of Salamander Hospitality, Sheila Johnson, is a prominent figure in the Middleburg community, and she felt that this would be bad for her image. The name of her company, Salamander Hospitality, is part of the name of the resort and this would be a direct negative tie to any future properties or communities the company would build.

Deceleration of a construction project is a very rare occurrence and is usually only done in extreme cases. There are numerous negatives aspects to the deceleration of a project schedule. When it comes to timeline and substantial completion dates, owners almost always want them to be earlier. They want to begin collecting rent from tenants or use the building themselves sooner rather than later. When a project is unintentionally delayed, it can cost the owner thousands of dollars a day. Clauses are sometimes built into the contract to make the contractor pay for lost profit if they don't finish on schedule. On this project, because the owner requested the delay, a different set of problems arose.

One of the problems that resulted from the deceleration of this project was the renegotiation of contracts between contractor and subcontractors. The two parties had to agree to when the required work will be completed and more importantly to the subcontractor, when they would be paid. The largest expense of the deceleration to the owner came from cost of the salaries of the Turner Construction project team for the additional 12 months. Temporary lighting, heating, and power are required for all the additional months and can be a large expense during the winter months.

For the revised schedule, most activities were not delayed rather their durations were extended over a longer period of time. The main exception to this schedule was the interior work. From January 2009 to November 2009, all interior work in the lodge was stopped. By the end of December 2008, a portion of the metal framing had been completed, and on January 1, 2009 the interior work was put on hold for 11 months. When it was stopped, some of the interior work had been completed, requiring heating and cooling during the shutdown period.

## Proposed Schedule

## See Appendix H for Proposed Schedule

I am proposing that instead of the revised schedule, where select activities were lengthened and interior work was put on hold, all activities be stopped for a period of approximately 10 months. Ten months instead of the full twelve months is chosen because certain activities should be completed prior to shutting down the site.

The shutdown of the site will occur between the shell and core completion dates for all the building areas and the start of the interior work. Interior metal framework will be included in the shell and core completion. The project shutdown will be for 43 weeks and occur between December 21, 2009 and October 18, 2010. The important dates of the proposed schedule can be seen below.

| Important Dates |  |  |  |
| :--- | :---: | :---: | :---: |
|  | Complete Shell and Core | Start Finish <br> Work | Complete Finish <br> Work |
| Lodge | $9 / 24 / 2009$ | $10 / 29 / 2010$ | $11 / 16 / 2011$ |
| Spa | $9 / 1 / 2009$ | $12 / 27 / 2010$ | $11 / 10 / 2011$ |
| Main Building | $12 / 18 / 2009$ | $10 / 19 / 2010$ | $11 / 30 / 2011$ |

The main building core and shell is completed last because it has the most complicated façade and requires additional installation time. Due to the extensive interior work, the finish work for all three areas take approximately 12 months to complete.

The interior finishing start and finish dates were modified to allow for the overlap of same trades across building areas. This was done so a subcontractor could work straight through and not have week breaks between the main building work and the spa work. An example of this overlapping is shown below with the Hanging/Taping/Finish of the drywall.

| Hang/Tape/Finish Drywall Dates |  |  |
| :--- | :---: | :---: |
|  | Start Date | Finish Date |
| Main Building | $12 / 21 / 2010$ | $3 / 15 / 2011$ |
| Spa | $2 / 1 / 2011$ | $2 / 21 / 2011$ |
| Lodge | $2 / 14 / 2011$ | $6 / 7 / 2011$ |

This analysis was done for all the finish work from Layout to MEP Trimout.

The main difference between my proposed schedule and the revised schedule is that the revised schedule still maintains the full Turner Construction project team on site. By removing the project team, the salaries and the office general conditions are eliminated. The temporary power can be reduced by $90 \%$, while the temporary lighting can be reduced by $80 \%$. As for the temporary heating, $30 \%$ is needed for the months December to March to keep the interior temperature at 40 degrees, and $0 \%$ is needed for April to November. In a finished space, humidity and temperature are a huge factor. Mold can grow on drywall, carpets, and curtains if the humidity is not correct. I eliminate this problem by placing the break in the schedule before any of these sensitive materials are installed. This was one of the contributing factors to choosing the dates of the 10 -month halt.

I consolidated related activities that were broken up across the revised schedule. The Ecostar Slate roof installation was originally scheduled for installation from August 30, 2010 to October 7, 2010 for the spa and from September 13, 2010 to November 5, 2010 for the main building. I moved it to immediately following the roof installation of spa and main building. This allows the roofing subcontractor to finish all his work instead of postponing approximately 30 work days.

Below is a summary of the general conditions saved during the 43 week break. See Appendix I for a detailed breakdown of the general conditions with the proposed schedule.

| General Conditions Savings |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: |
| Description | Cost |  |  |  |
| Field Personnel | $\$ 423,765.00$ |  |  |  |
| General Expenses | $\$ 24,700.00$ |  |  |  |
| Temporary Utilities | $\$ 47,155.12$ |  |  |  |
|  |  |  | Sub-Total | $\$ 495,620.12$ |
|  | Location Factor |  |  |  |
|  | Total |  |  |  |
| $\$ 486,698.96$ |  |  |  |  |

Offsetting some of the savings are additional security expenditures. Security is needed on a full time basis to protect against theft and vandalism. A single security guard on two 12 -hour shifts will be required. The total cost of security for 43 weeks is $\$ 234,354.30$. See Appendix J for a detailed breakdown of security. This offsets approximately $48 \%$ of the overall general conditions savings.

## Recommendation

The following chart summarizes the overall general conditions savings and additional cost.

| Overall Savings |  |  |  |
| :--- | :--- | :--- | :---: |
| General Conditions Savings | $\$$ | $486,698.96$ |  |
| Temporary Security Cost | $\$$ | $234,354.30$ |  |
|  |  |  |  | Total Savings

The overall savings from my proposal are approximately a quarter of a million dollars. On a project that was heavily value engineered before and during construction, this amount is significant and this timeline should be considered as an alternative to the revised schedule. The 10 -month break allows the owner to consider any other value engineering topics without the time restrictions. Turner Construction also benefits from this because it allows them to rearrange manpower and place the current staff on other projects.

## Analysis II: Guest Room Lighting Redesign

## Introduction

As with most hotels and resorts, a large amount of energy is wasted because occupants leave the lights on when they are out of the room. The Salamander Resort and Spa has 168 guest rooms and the likelihood of everyone turning off unneeded lights is very low. When in the room, occupants also tend to use more light than is necessary, having multiple light fixtures on at once. With the introduction of compact fluorescent and LED lights in recent years, the ability to significantly reduce energy bills has increased. Specialized lighting control systems also increase efficiency and reduce cost.

The resort has 168 guest rooms in seven different layout types, king, ADA king, double queen, ADA double queen, junior suite, executive suite, and presidential suite. The breakdown of the room types is shown below.

|  | Number of Rooms |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: |
| Room Type | Ground Floor | 1st Floor | 2nd Floor | 3rd Floor | Total |
| King | 22 | 24 | 29 | 27 | 102 |
| ADA King | 1 | 1 | 1 | 1 | 4 |
| Double Queen | 12 | 13 | 8 | 8 | 41 |
| ADA Double Queen | 2 | 1 | 1 | 0 | 4 |
| Junior Suite | 3 | 3 | 3 | 3 | 12 |
| Executive Suite | 1 | 1 | 1 | 1 | 4 |
| Presidential Suite | 0 | 0 | 0 | 1 | 1 |

For this analysis, the room types are broken into two layouts, king/queen (151 rooms) and suites (17 rooms). The lighting layout of the junior suites is used for all the suites.

## Current System

The current lighting system for The Salamander Resort and Spa is highly customized. Halogen lamps were chosen for their increased life expectancy and high output over normal incandescent lamps. The guest rooms are predominantly halogen downlights that utilize Par20 lamps. The ceiling and wall fixtures for the space are not specified but designed wattage is given. I will assume these fixtures use Par16 halogen lamps. The number of lamps needed is determined by the designed wattage. The lamp specifications for the Par20 and Par16 lamps can be seen below:

## Final Report

| Par20 Halogen |  |
| :--- | :---: |
| Approx. Lumens | 570 |
| Average Rated Life (hr) | 2500 |
| Beam Type | Flood |
| Beam Angle | 25 deg. |
| Diameter (in) | 2.5 |
| Diameter (mm) | 63.5 |
| Filament | CC-8 |
| Maximum Overall Length (in) | 3.125 |
| Maximum Overall Length (mm) | 79.3 |
| Nominal Voltage (V) | 120 |
| Nominal Wattage (W) | 50 |
| Price | $\$ 5.50$ |


| Par16 Halogen |  |
| :--- | :---: |
| Approx. Lumens | 450 |
| Average Rated Life (hr) | 2500 |
| Beam Type | Flood |
| Beam Angle |  |
| Diameter (in) |  |
| Diameter (mm) |  |
| Filament |  |
| Maximum Overall Length (in) |  |
| Maximum Overall Length (mm) | 120 |
| Nominal Voltage (V) | 45 |
| Nominal Wattage (W) | $\$ 7.95$ |
| Price |  |

It is important to note the lumens, average rated life, nominal wattage, and price for both lamps. The Par20 lamps produce 570 lumens while the Par16 puts out slightly less, 450 lumens. The average rated life is 2,500 hours which is typical of most halogen lamps. The dimensions of the Par16 lamp will be determined when the fixture is designed. The Par20 costs $\$ 5.50 / \mathrm{lamp}$ and the Par16 is $\$ 7.95 / \mathrm{lamp}$. The price is for the lamp alone and does not include installation costs. The wattage for both lamps is around 50 watts. This value is similar to that of a comparable incandescent lamp.

The current fixture schedule is shown below for both the typical king/queen room and junior suite:

| Typical Double Queen/King Guestroom (151 Rooms) |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Type | Description | Quantity of Fixtures | Per Lamp |  |  | Quantity of Lamps | Volt-Amps |
|  |  |  | Voltage | Watts | Amps |  |  |
| AM | Adj. Downlight | 2 | 120 | 50 | 0.42 | 2 | 100 |
| AN | Downlight | 2 | 120 | 50 | 0.42 | 2 | 100 |
| AP | Shower Rated Downlight | 2 | 120 | 50 | 0.42 | 2 | 100 |
| AR | Downlight | 2 | 120 | 50 | 0.42 | 2 | 100 |
|  |  |  |  |  |  |  |  |
| GD1 | Decorative Wall Fixture | 1 | 120 | 45 | 0.38 | 1 | 45 |
| GD2 | Decorative Ceiling Fixture | 1 | 120 | 45 | 0.38 | 3 | 135 |
| GD3 | Decorative Wall Fixture | 2 | 120 | 45 | 0.38 | 2 | 90 |
| GD4 | Decorative Ceiling Fixture | 1 | 120 | 45 | 0.38 | 2 | 90 |
|  |  |  |  |  |  | Total/Room | 760 |

There are a total of 8 downlight fixtures with one lamp in each. There are also 5 decorative ceiling and wall fixtures that combine for a total of 8 lamps. GD2 and GD4 have multiple lamps because the designed wattage is greater than the wattage of one lamp. Three lamps are put in the GD2 ceiling fixture and 2 in the GD4 ceiling fixture. The number of volt-amps was calculated for each fixture and then summed for the whole room. The total number of volt-amps for one king/queen room is 760 VA.

| Typical Junior Suite (17 Rooms) |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Type | Description | Quantity of Fixtures | Per Lamp |  |  | Quantity of Lamps | Volt-Amps |
|  |  |  | Voltage | Watts | Amps |  |  |
| AM | Adj. Downlight | 2 | 120 | 50 | 0.42 | 2 | 100 |
| AN | Downlight | 2 | 120 | 50 | 0.42 | 2 | 100 |
| AP | Shower Rated Downlight | 2 | 120 | 50 | 0.42 | 2 | 100 |
| AR | Downlight | 2 | 120 | 50 | 0.42 | 2 | 100 |
|  |  |  |  |  |  |  |  |
| GD1 | Decorative Wall Fixture | 1 | 120 | 45 | 0.38 | 1 | 45 |
| GD2 | Decorative Ceiling Fixture | 1 | 120 | 45 | 0.38 | 3 | 135 |
| GD3 | Decorative Wall Fixture | 0 | 120 | 45 | 0.38 | 0 | 0 |
| GD4 | Decorative Ceiling Fixture | 2 | 120 | 45 | 0.38 | 4 | 180 |
|  |  |  |  |  |  | Total/Room | 760 |

The junior suite fixture schedule is similar to the king/queen rooms. The only difference is the number of decorative wall and ceiling fixtures. Despite this difference, the number of lamps is unchanged. When the volt-amps are summed across all lamps in the junior suite the room total is 760 VA.

For each room type the volt-amps are the same, 760 VA . This was applied to all the rooms in the lodge and converted to kW to get the total energy use.


For all the king/queen rooms in the lodge the total energy use is 114.76 kW , while the suites consume 12.92 kW . I will compare costs below.

## Proposed System

The proposed system will replace all halogen lamps with LED's and install a control system that will turn off the guest room's lights when no one is present. The two manufacturers used are EarthLED for the lamps and Messerschmitt for the control.

LED lamps are chosen over compact fluorescent because CFL's contain mercury and must be treated as hazardous waste upon disposal. Modern LED lamps are able to replicate the light emitted by incandescent and halogen bulbs at a significantly lower wattage. This increased efficiency leads to direct energy cost savings. Other benefits of LED lamps include instant full brightness upon startup, no output of ultraviolet light, reduced maintenance cost due to long life, and less heat production. The main disadvantage of LED lamps is the relatively high bulb cost. This can deter many of potential buyers who are unfamiliar with the substantial energy savings.

Two different types of LED bulbs will be installed, both of which are manufactured by EarthLED. The lamps are direct replacement retrofits that do not require any addition equipment for installation. The lamp to be installed in the downlights is "EarthLED Lumiselect Par20/R20 Dimmable LED". The "EarthLED Lumiselect Par16/R16 Dimmable LED" will be installed in the decorative wall and ceiling fixtures. Lamp specifications can be seen below.

| EarthLED LumiSelect PAR20/R20 Dimmable LED |  |
| :--- | :---: |
| Approx. Lumens | 450 |
| Average Rated Life (hr) | 50,000 |
| Beam Type | Flood |
| Beam Angle | 90 |
| Diameter (in) | 2.91 |
| Diameter (mm) | 74 |
| Filament |  |
| Maximum Overall Length (in) | 4.01 |
| Maximum Overall Length (mm) | 102 |
| Nominal Voltage (V) | 120 |
| Nominal Wattage (W) | 9 |
| Comparable Wattage (Incandescent) | $50-60$ |
| Price | $\$ 90$ |


| EarthLED LumiSelect Par16/R16 Dimmable LED |  |
| :--- | :---: |
| Approx. Lumens | 300 |
| Average Rated Life (hr) | 50,000 |
| Beam Type |  |
| Beam Angle | 90 |
| Diameter (in) | 6.36 |
| Diameter (mm) |  |
| Filament | 4.25 |
| Maximum Overall Length (in) | 108 |
| Maximum Overall Length (mm) | 120 |
| Nominal Voltage (V) | 6 |
| Nominal Wattage (W) | 50 |
| Comparable Wattage (Incandescent) | $\$ 70$ |
| Price |  |

The three most important differences between the halogen and LED lamps are average rated life, nominal wattage, and price. The average LED lamp is rated for 50,000 hours, 20 times longer than the specified halogen lamp while using less than a fifth of the energy. The lumen output is slightly less than the halogen counterpart but EarthLED claims the usable light output is similar
to that of a 50 watt incandescent lamp. The LED's cost significantly more than the halogen lamps and the payback period will be analyzed later in this section. A factor that determines which lamp is chosen is the ability of dimming, as the original halogen lamps are dimmable.

One of the largest energy drains in the hospitality industry is hotel guests leaving lights and electronics on in the room when they are not present. In most hotel settings, guests are absent from their room for extended periods of time during the day. To combat this problem, this analysis is proposing the use of a stand-alone guest occupancy key card system. Upon entering the room, the guest places the hotel room key in a card reader. When the card is in the reader, electricity flows to all connected lights and electronics. When the guest leaves, they take the key card and all connected lights are turned off to save energy. This is similar to many hotel lighting control systems found in Asia. For this analysis, it is assumed that the guests are absent from their rooms between the hours of 10 am and 4 pm .

## Cost Analysis - Electrical Breadth Analysis

## Energy Cost

On initial inspection, it might seem that the initial cost of the LED lamps far outweighs the long term energy cost savings, but one has to remember that it is not only the energy savings from the lamp but also the lighting control system that keeps them on for about half the time of the halogens. The chart below shows the energy cost per year broken down by lamp type. The quantity of lamps, 1344, is the total number lamps of each type in the guest lodge at any one point. Calculating the energy use of the halogen lamps per kWh assumes that the lights will not be turned off during the unoccupied or occupied time frame. The proposed system with LED lamps will shut off during the unoccupied time frame, 10 am to 4 pm . The cost per kWh in Middleburg, Va is $\$ 0.10$.

| Current Energy Use (Halogen) |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lamp | Quantity of Lamps | Watts /Lamp | Total Watts | Total kW | Unoccupied 10AM 4PM | Occupied 7 hrs. | $\begin{aligned} & \text { kWh } \\ & \text { /day } \end{aligned}$ | $\begin{gathered} \$ \\ / k W h \end{gathered}$ | $\begin{gathered} \$ \\ \text { /day } \end{gathered}$ | \$/year |
| 50PAR20H/FL25 | 1344 | 50 | 67200 | 67.2 | 403.2 | 470.4 | 873.6 | 0.10 | 87.36 | 31886.40 |
| 45PAR16/FL | 1344 | 45 | 60480 | 60.5 | 362.9 | 423.4 | 786.2 | 0.10 | 78.62 | 28697.76 |


| Proposed Energy Use (LED) |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lamp | Quantity of Lamps | Watts /Lamp | Total Watts | Total kW | Unoccupied 10AM 4PM | Occupied 7 hrs. | $\begin{aligned} & \text { kWh } \\ & \text { /day } \end{aligned}$ | $\begin{gathered} \$ \\ / \mathrm{kWh} \end{gathered}$ | $\begin{gathered} \$ \\ \text { /day } \end{gathered}$ | \$/year |
| EarthLED <br> LumiSelect <br> PAR20/R20 <br> Dimmable LED | 1344 | 9 | 12096 | 12.1 | 0.0 | 84.7 | 84.7 | 0.10 | 8.47 | 3090.53 |
| EarthLED <br> LumiSelect <br> Par16/R16 <br> Dimmable LED | 1344 | 6 | 8064 | 8.1 | 0.0 | 56.4 | 56.4 | 0.10 | 5.64 | 2060.35 |


| Existing kWh/day | Existing \$/year |  |
| :---: | ---: | ---: |
| 1659.84 | $\$$ | $60,584.16$ |


| Proposed kWh/day | Proposed $\$ /$ year |
| :---: | ---: |
| 141.12 | $\$$ |
| $5,150.88$ |  |

The annual savings on energy alone by installing the lighting control system and alternate lamps is over 55 thousand dollars. Before the recommendation can be made to implement this system, one has to look to factor in the bulb and maintenance costs.

## Halogen Replacement Cost

With a 2,500 hour lamp life and 13 hours of use per day, one halogen lamp will last approximately 6.5 months. This comes out to 1.9 halogen lamps per year, per fixture. Maintenance costs come into play when replacing the bulbs. Assume 10 lamps can be replaced every hour and at $\$ 20 / \mathrm{hr}$, the cost per lamp is an additional $\$ 2$. The table below shows the cost per year to replace halogen lamps.

| Halogen Replacement Cost |  |  |  |  |
| :--- | :--- | :---: | :---: | :---: |
| Room Type | Lamp Type | Lifetime <br> Lamp Hours | Hrs. in <br> use/year <br> (Fixture) | Lamps/year |
| Typ. King/Queen | 50PAR20H/FL25 | 2500 | 4745 | 1.90 |
| Typ. King/Queen | 45PAR16/FL | 2500 | 4745 | 1.90 |
|  |  |  |  |  |
| Typ. Suite | 50PAR20H/FL25 | 2500 | 4745 | 1.90 |
| Typ. Suite | 45PAR16/FL | 2500 | 4745 | 1.90 |


| Halogen Replacement Cost |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Room Type | $\$ /$ Lamp + <br> Install* | \$/Year/Lamp | Lamps/Room | Rooms | \$/Year Total |  |
| Typ. King/Queen | $\$$ <br> 7.50 | $\$$ | 14.24 | 8 | 151 | $\$ 17,195.88$ |
| Typ. King/Queen | $\$$ <br> 9.95 | $\$$ | 18.89 | 8 | 151 | $\$ 22,813.20$ |
|  | $\$$ <br> 7.50 | $\$$ | 14.24 | 8 | 17 | $\$$ <br> $1,935.96$ |
| Typ. Suite | $\$$ <br> 9.95 | $\$$ | 18.89 | 8 | 17 | $\$$ <br> $2,568.37$ |
| Typ. Suite |  |  |  |  |  |  |

*Install Cost: 10 per hour at $\$ 20 / \mathrm{hr}=\$ 2 / \mathrm{lamp}$

| Total Cost/year | $\$ 44,513.41$ |
| :--- | :--- |

The total cost per year to replace the halogen bulbs comes out to slightly over 44 thousand dollars. This value includes the initial investment of purchased lamps.

## Initial Investment

The next thing to consider is the initial investment of the proposed system. With a 50,000 hour lamp life and 7 hours of use per day, one LED lamp will last approximately 17 years. The following chart shows the cost to buy and install the LED lamps in all the guest rooms. Unlike the halogen replacement chart, this is a one-time initial cost calculation as the next time the lamps will need to be replaced is after 17 years.

| Lamp Type | Lamps <br> /Room | Rooms | Lamps | $\$ /$ Lamp + <br> Install | Cost (\$) |
| :--- | :---: | :---: | :---: | :---: | :---: |
| EarthLED LumiSelect PAR20/R20 <br> Dimmable LED | 8 | 168 | 1344 | $\$$ <br> 92.00 | $\$$ <br> $123,648.00$ |
| EarthLED LumiSelect Par16/R16 <br> Dimmable LED | 8 | 168 | 1344 | $\$$ <br> 72.00 | $\$$ <br> $96,768.00$ |

The key card system plus installation will be $\$ 100$ per room, which totals to $\$ 16,800$ for all the guest rooms.

| Total Initial <br> Proposed Cost |
| :---: | :---: |
| $\$ 237,216.00$ |

## Operating Cost

The total proposed lighting system including lamps, install, and control system cost comes out to a little over a quarter of a million dollars. The proposed initial and energy cost is more than the halogen annual replacement and energy cost. The final piece of information to look at is the payback period for the existing versus the proposed.

|  |  | Operating Costs/year |  |
| :--- | :---: | :---: | :---: |
|  | Initial Investment | Replacement Cost^ | Energy cost |
| Existing | $0^{*}$ | $\$ 44,513.41$ | $\$ 60,584.16$ |
| Proposed | $\$ 237,216.00$ | 0 | $\$ 5,150.88$ |

*included in annual replacement cost
^per year for 17 years
To calculate the payback period to recoup the initial investment of the new system one must add the replacement and energy cost of the existing system, subtract from that the energy cost of the proposed system and then divide that number into the proposed initial investment. The final payback period comes out to 2.37 years.

## Recommendation

The initial investment of the proposed system is significantly higher than the current system, but with $\$ 55,000$ annual savings in energy and $\$ 44,000$ less per year in replacement costs, the system has a payback period of 2.37 years. Following the payback period, the annual savings is approximately $\$ 100,000$ for the following 15 years, or until the LED lamps burn out. Replacing all the LED lamps costs $\$ 220,416.00$, this is more than made up for in the 15 previous years of energy and replacement savings.

## Analysis III: Water Management

## Native Plants

The Salamander Resort and Spa employs an immense number of different trees, shrubs, vines, perennials and annuals. This is done to create a diverse and unique look around the building. One objective of this analysis is to create a similar look to the originally designed plants but replace them with native plant species. Using native plants has many benefits, the largest being that they are able to survive on natural rainfall and don't need an irrigation system to prosper. Native species are plants that are present in the region in which they have evolved. They are accustomed to the climate, rainfall, soil, frost, and interactions with other species. Native plants have developed a special set of characteristics that allow them to prosper in the current region without the use of fertilizers or pesticides. They also have the ability to match the quality and aesthetics of invasive and exotic plants while surpassing them in durability, resistance to insects, and disease.

One of the most important characteristics of native species in relation to this analysis is the ability to survive and prosper on the natural rainfall. I have chosen to just focus on replacing shrubs because they draw the most water in the areas where irrigation is placed. The vast majority of trees lie outside of the reach of the irrigation system because they are more tolerant of drought conditions. The perennials and annuals placed around the site provide the Resort with a very unique look that is hard to replicate. They also require the least amount of water on a per plant basis. One issue with performing a water savings analysis is that it is nearly impossible to quantify the actual savings.

According to the Virginia Department of Conservation and Recreation there are close to sixty different species of native shrubs in the state of Virginia. Below is a list of the proposed shrubs along with the native alternative.

| Quantity | Proposed | Native Replacement |
| :---: | :---: | :---: |
| 277 | Fothergilla | Buttonbush |
| 71 | Cherry Laurel | Virginia Sweetspire |
| 74 | Otto Luyken Cherry Laurel | Henry Garnet's Sweetspire |
| 64 | San Jose Holly | Red Chokeberry |
| 19 | Korean Spice Viburnum | Arrowwood Viburnum |
| 5 | Shasta Doublefile Viburnum | Possumbaw Viburnum |
| 91 | Nandina | Inkberry |
| 27 | Vernal Witchhazel | Common Witchhazel |

See Appendix K for a detailed Shrub Replacement Plan
I focused the replacement process on the shrubs which appear at least 25 times as they provide the greatest water savings impact. Virginia and Henry Garnet's Sweetspire already appeared on site so I split the two species of Cherry Laurel between them. There is very little difference between the four species of Viburnums, so I replaced the least frequently occurring plants, Korean Spice and Shasta Doublefile, with the Arrowwood and Possumbaw.

## Climate

Virginia has a humid and sub-tropical climate. Very hot and humid summers yet cool winters that often produce frost. There are four distinct seasons each year. During the summer months, short rain squalls are common, while the month of May receives the most precipitation. Annual average temperatures fall between 45 and 50 degrees Fahrenheit. The chart below shows the average monthly rainfall in Northern Virginia.

| Average Rainfall in Northern Virginia (inches) |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sept | Oct | Nov | Dec | Year |
| 3.2 | 2.8 | 3.7 | 3.3 | 4.3 | 4.0 | 4.4 | 3.5 | 3.7 | 3.3 | 3.3 | 2.9 | 40.6 |

One of the factors in determining what kind of plant species will survive in a given area is the USDA Frost Zone map. This map classifies each zone, number 1-10, of the US based on minimum temperature and earliest and latest dates of possible frosting. This is called plant hardiness, and the plant hardiness for Middleburg, Virginia is Zone 6. Zone 6 plants have the ability to withstand temperature as low as -10 degrees F . The average date of the first frost is September 1 - September 30, while the average day of the last frost is between May 1 and May 30.


The lower the zone number, the higher the hardiness of the plant species. It is important to consider the optimal zone of each plant because it determines whether it will survive cold winters or hot summers. This is another reason why native plants should be considered in plantings because they are in the given zone and are able to survive the climate.

## Current Irrigation System

The main supply for the currently proposed irrigation system comes from a pump located at an existing pond to the northwest of the building. The supply runs from that point, in a 3" PVC mainline, to several remote valves in each of the three areas. $1.5 "$ to $3 " \mathrm{PVC}$ piping is used to distribute the water from the mainline to the irrigation fixtures. Immediately around the building, drip tubing is used for watering while the remaining portions have 4", 6 " of 12 " sprayhead fixtures. Quick couplers are attached to the main line that surrounds the culinary garden to allow for specialized hookups. A PVC sleeve is installed around the piping that goes underneath concrete walkways or driveways to prevent structural failure. A wireless rain sensor is located on the southeast side of the building to shut off the irrigation system if there has been enough rainfall.

## Proposed Irrigation System

See Appendix L for full specifications

The proposed rainwater collection system is manufactured by Snyder Industries. This project will utilize a series of below ground cistern tanks in either the 1200 or 1700 gallon size. The tanks will be placed in series with one another to obtain the capacity required in each of the three irrigation areas seen below.


They will be placed in non traffic areas to reduce the possibility to structural failure. This system will tap into the current piping and fixtures and replace the pond pumping station. The location of tanks can be seen here:


The tanks have a pump inside that will be used to bring the rainwater to the surface. From there, the tanks are positioned such that the irrigation can mainly be gravity-fed.

## Rainwater Collection Potential

The use of rainwater is an economical alternative to public water and is ideal for irrigation systems. This system will also reduce the water extraction from the onsite pond that is currently being tapped for the irrigation. The preservation of this pond during the hot summer months will enhance the appeal of the surrounding area. Rainwater is very low in minerals so it is an excellent source of irrigation for sensitive plants and flowers. It is not regulated by the municipality so in a time of drought, when water restrictions are in place, the rainwater collected from earlier months can protect your investments in landscaping. By utilizing rainwater collections tanks, storm water runoff is reduced, thus eliminating the danger of soil erosion, water drain overflow, and water pollution.

Rainwater can be collected from almost any surface, but bare rooftops provide the easiest collection and the water usually contains the least amount of contaminants and chemicals. Not all the water that strikes a rooftop can be collected because a portion is lost to evaporation, blowing wind, leaks, and overflowing gutters. The collectable water that can be obtained after the loss factors can be summarized in the following equation:

Collectable rainwater $($ gallons $)=.5 \mathrm{x}$ rainfall (inches) x area $($ square feet $)$

The Salamander Resort and Spa has approximately 28,600 square feet of roof area, which when calculated along with an average of 42.4 inches of rain annually comes out to 606,320 gallons/year of potential rainwater collection. As shown in the later analysis, not all of the potential collection is necessary.

Along the east coast of the US, the rainfall is relatively evenly distributed throughout the year which makes for easy rainwater collection and distribution. This also helps in simplifying the sizing of the water tanks as there is no need to worry about over sizing to account for drier months.

## Water Requirements - MEP Breadth Analysis

Determining water use for an irrigation system depends heavily on the proportion of native plants present. According to Snyder Industries, temperate-climate plants, like the ones found in Northern Virginia, need about 1-inch of rainfall per week to survive. The plants in the local region obtain between 3.0 and 3.5 inches of rainfall a month, which translates to between .75 and
.88 inches a week. For my calculations, I will assume .75 inches per week. The factor of .6 is used in the following equation to represent water use of temperate-climate plants.

$$
\text { Gallons/week needed }=.6 \times(\text { square feet })
$$

Irrigation area 1, located to the north of the lodge is the largest of the three irrigation zones. The total area is $28,000 \mathrm{ft}^{2}$. The proposed irrigation system will be designed as a supplement to natural rainfall.
The above equation is used to determine the overall water requirement for this area.
$.6 \times 28,000 \mathrm{ft}^{2}=16,800 \mathrm{gal} / \mathrm{wk}$ needed

In order to accurately size the rainwater collection tanks, the amount of natural rainfall in the irrigation area has to be determined. This calculation is done using the .75 inches per week assumption.
$.75 \mathrm{in} /$ week $=108 \mathrm{in}^{3} / \mathrm{ft}^{2}=.47 \mathrm{gal} / \mathrm{ft}^{2}$
$.47 \times 28,000 \mathrm{ft}^{2}=13,160 \mathrm{gal} / \mathrm{wk}$ natural rainfall

The difference between the needed and natural rainfall, gallons per week is the determining factor for sizing the collection tank.
$16,800-13,160=3,640 \mathrm{gal} / \mathrm{wk}$

These same calculations are repeated for the two other irrigation areas and the findings are summarized in the following table:

| Area | Square Footage | Needed (gal/wk) | Natural (gal/wk) | Difference (gal/wk) |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 28,000 | 16,800 | 13,160 | 3,640 |
| 2 | 12,825 | 7,700 | 6,030 | 1,670 |
| 3 | 17,100 | 10,260 | 8,040 | 2,223 |

The gallons/week difference that is needed for the proposed irrigation system must be matched up with a roof area that will adequately supply the cistern tanks. See Appendix M for the assigned roof collection area. The following table summarizes the cistern sizing:

| Area | Rainwater <br> Needed | Roof Area <br> $\left(\mathrm{ft}^{\wedge} 2\right)$ | Rainwater <br> Collected <br> $(\mathrm{gal} / \mathrm{wk})$ | Tanks |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 3,640 | 9540 | 4220 | (4) 1200 gal |
| 2 | 1,670 | 4831 | 2130 | (2) 1200 gal |
| 3 | 2,223 | 8190 | 3620 | $(2) 1700 \mathrm{gal}$ |

I conservatively sized the tanks to err on the high side in order to account for droughts and dry spells. Area 3, on the east side of the building, is sized extra large because the garden will need additional manual watering.

## Constructability and Schedule Impact

With the introduction of more native plant species on the project, it lessens the impact of the schedule. Native species are more readily found at local nurseries than the more exotic types. There are dozens of nurseries within 50 miles of Middleburg, Virginia that would be able to supply all the necessary native plants for the project. The larger and more exotic plants that the project requires, the longer the lead time is needed to get them to the site. Care should be taken to find a nearby nursery that can accommodate the size and scope of The Salamander Resort and Spa. It will probably be necessary to find numerous suppliers to fill the large and diverse order.

The Virginia Department of Conservation and Recreation gives information about the purchasing and selection of native plants. Due to the seasonal availability of many plants, ordering all the required plants at once can be quite difficult. Contact with the nurseries will be necessary to adequately gauge the amount of lead time required. Because of this problem, it is difficult to determine the impact on the project schedule. If the correct research is done, the project schedule has the potential to decrease in length because of the decrease in shipping distance.

A total of 8 cistern tanks will be placed in three different locations on site. Each set of tanks will require a concrete pad for the base. This can be performed when the foundations for the building are being placed, and will add one day to the schedule. In order to prevent the tank from floating in the soil, they must be strapped down with hooks cast into the concrete pad. It can only be backfilled once the tanks have been strapped down. The tanks are to be installed per Snyder Industries instructions.

In order to accommodate the additional rainwater, the gutters around the roof collection area will be increased by 2 inches. The main downspout leading to the tanks will be upgraded to a 6 -inch pipe. The gutters and downspouts outside of the roof collection area will not change in size.

The fixtures and piping will not be changed so there is not a schedule impact. In place of the pond pump station and piping there will be three hook-ups to the cistern rainwater collection tanks. These activities have off setting durations so the schedule is not impacted.

## Cost Analysis

The most significant additional cost is the price of the Snyder collection tanks. A summary of these costs is seen below:

| Tank Size | Quantity | Price/each | Cost |
| :---: | :---: | :---: | :---: |
| 1200 Gallons | 6 | $\$ 2,900$ | $\$ 17,400$ |
| 1700 Gallons | 2 | $\$ 3,600$ | $\$ 7,200$ |
|  |  | Total | $\$ 24,600$ |

The savings achieved from this proposal comes from the loss of the pond pump station, concrete pad, and subsequent piping to the irrigation system. The cost breakdown of the pump station is as follows:

| Equipment | Price (\$) |  |  |
| :--- | :---: | :---: | :---: |
| Pump, Goulds 3656/Motor 3600 RPM | $\$ 2,400$ |  |  |
| 1 kVa Transformer | $\$ 320$ |  |  |
| Variable Frequency Drive, ACS550 | $\$ 2,000$ |  |  |
| Exhasut Fan, 1320 CFM | $\$ 80$ |  |  |
| Pressure Transducer | $\$ 150$ |  |  |
| GB6 Electronic Controller, Tekleen | $\$ 1,000$ |  |  |
| Backwash Filter | $\$ 300$ |  |  |
| Total |  |  | $\$ 6,250$ |

The additional cost of this proposal, relative to the existing plan is $\$ 18,350$. This ignores some cost components in each system. For this rough estimate I am assuming the existing concrete pad, piping from the pump station to the system hookup cancel out the proposed cost of the cistern hookups, and concrete pads for the tanks. These costs would more or less cancel each other out and be relatively small in comparison to $\$ 18,350$.

The on-site pond is located quite a distance away from the building and is significantly lower in elevation. This poses two problems, first is the distance that the water needs to be pumped and second, the amount of electricity the pump uses to get it up the hill to the irrigation system. With the water collection system in place, the water tanks will be located directly underneath the irrigation areas. This will reduce the overall distance the water needs to travel from the source to
the irrigation system. Each tank will have a small pump that will distribute the rainwater. These motors only need to pump the water 3 vertical feet which will cut down on the energy use.

## Conclusion and Recommendation

The use of native plants in landscaping can have a significant positive impact on water use without drastically changing the aesthetics. Native plants are accustomed to the climate and conditions of the location and are hardier and more likely to survive harsh conditions than that of non-native and exotic plants. It is important to note both the aesthetic characteristics and hardiness of a plant when replacing it with an alternative.

Some of the non-native species in the existing plan have close relatives that are native to the region. This made changing the plant species a trivial task and was confusing as to why the native plant was not specified in the first place. For the remaining non-native plants, focus was placed on the species that are used over 25 times.

The estimated additional cost of $\$ 18,350$ is a relatively small cost for increased efficiency and sustainability. It is recommended that The Salamander Resort and Spa implement the proposed rainwater collection system and native plant redesign.

## Appendix A: Site Plan of Existing Conditions



## Appendix B: Site Layout Planning



## Appendix C: D4 Cost Estimate

## Statement of Probable Cost

| Salamander Resort and Spa - Mar 2009 - VA - Arlington |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Prepared By: | Paul Roberts |  | Prepared For: | Paul Roberts |  |
|  | Building Sq. Size: <br> Bid Date: <br> No. of floors: No. of buildings: Project Height: 1st Floor Height: 1st Floor Size: | Fax: <br> 229213 <br> 4 <br> 1 <br> 10.25 |  | Site Sq. Size: <br> Building use: <br> Foundation: <br> Exterior Walls: <br> Interior Walls: <br> Roof Type: <br> Floor Type: <br> Project Type: | Fax: <br> 66135 <br> Hotel/Motel <br> CON <br> STU <br> GYP <br> SLA <br> WOD <br> NEW |  |
| Division |  |  | Percent |  | Sq. Cost | Amount |
| 00 | Bidding Require Bidding Requ | ents rements | $\begin{aligned} & 7.22 \\ & 7.22 \end{aligned}$ |  | $\begin{aligned} & 9.60 \\ & 9.60 \end{aligned}$ | 2,200,317 $2,200,317$ |
| 01 | General Require General Req | ents irements | $\begin{aligned} & 3.75 \\ & 3.75 \end{aligned}$ |  | $\begin{aligned} & 4.99 \\ & 4.99 \end{aligned}$ | $\begin{aligned} & 1,144,345 \\ & 1,144,345 \end{aligned}$ |
| 02 | Site Work Site Work |  | $\begin{aligned} & 4.08 \\ & 4.08 \end{aligned}$ |  | $\begin{aligned} & 5.42 \\ & 5.42 \end{aligned}$ | $\begin{aligned} & 1,242,343 \\ & 1,242,343 \end{aligned}$ |
| 03 | Concrete Concrete |  | $\begin{aligned} & 21.00 \\ & 21.00 \end{aligned}$ |  | $\begin{aligned} & 27.92 \\ & 27.92 \end{aligned}$ | $\begin{aligned} & 6,400,073 \\ & 6,400,073 \end{aligned}$ |
| 04 | Masonry Masonry |  | $\begin{aligned} & 6.09 \\ & 6.09 \end{aligned}$ |  | $\begin{aligned} & 8.09 \\ & 8.09 \end{aligned}$ | $\begin{aligned} & 1,855,012 \\ & 1,855,012 \end{aligned}$ |
| 05 | Metals Metals |  | $\begin{aligned} & 1.56 \\ & 1.56 \end{aligned}$ |  | $\begin{aligned} & 2.07 \\ & 2.07 \end{aligned}$ | $\begin{aligned} & 475,578 \\ & 475,578 \end{aligned}$ |
| 06 | Wood \& Plastics Wood \& Plas |  | $\begin{aligned} & 4.78 \\ & 4.78 \end{aligned}$ |  | $\begin{aligned} & 6.36 \\ & 6.36 \end{aligned}$ | $\begin{aligned} & 1,457,405 \\ & 1,457,405 \end{aligned}$ |
| 07 | Thermal \& Moist Thermal \& M | re Protection isture Protection | $\begin{aligned} & 1.91 \\ & 1.91 \end{aligned}$ |  | $\begin{aligned} & 2.54 \\ & 2.54 \end{aligned}$ | $\begin{aligned} & 581,897 \\ & 581,897 \end{aligned}$ |
| 08 | Doors \& Window Doors \& Win | ows | $\begin{aligned} & 6.39 \\ & 6.39 \end{aligned}$ |  | $\begin{aligned} & 8.50 \\ & 8.50 \end{aligned}$ | $\begin{aligned} & 1,947,182 \\ & 1,947,182 \end{aligned}$ |
| 09 | Finishes Finishes |  | $\begin{aligned} & 11.38 \\ & 11.38 \end{aligned}$ |  | $\begin{aligned} & 15.13 \\ & 15.13 \end{aligned}$ | $\begin{aligned} & 3,467,559 \\ & 3,467,559 \end{aligned}$ |
| 10 | Specialties Specialties |  | $\begin{aligned} & 0.35 \\ & 0.35 \end{aligned}$ |  | $\begin{aligned} & 0.47 \\ & 0.47 \end{aligned}$ | $\begin{aligned} & 107,688 \\ & 107,688 \end{aligned}$ |
| 11 | Equipment Equipment |  | $\begin{aligned} & 0.09 \\ & 0.09 \end{aligned}$ |  | 0.12 0.12 | 26,443 26,443 |
| 12 | Furnishings Furnishings |  | $\begin{aligned} & 0.05 \\ & 0.05 \end{aligned}$ |  | $\begin{aligned} & 0.06 \\ & 0.06 \end{aligned}$ | $\begin{aligned} & 13,973 \\ & 13,973 \end{aligned}$ |
| 13 | Special Construc Special Cons | ion ruction | $\begin{aligned} & 0.68 \\ & 0.68 \end{aligned}$ |  | $\begin{aligned} & 0.90 \\ & 0.90 \end{aligned}$ | $\begin{array}{r} 206,514 \\ 206,514 \end{array}$ |
| 14 | Conveying Syste Conveying S |  | $\begin{aligned} & 2.49 \\ & 2.49 \end{aligned}$ |  | $\begin{aligned} & 3.31 \\ & 3.31 \end{aligned}$ | $\begin{array}{r} 758,084 \\ 758,084 \end{array}$ |
| 15 | Mechanical Mechanical |  | $\begin{aligned} & 19.61 \\ & 19.61 \end{aligned}$ |  | $\begin{aligned} & 26.08 \\ & 26.08 \end{aligned}$ | $\begin{aligned} & 5,978,605 \\ & 5,978,605 \end{aligned}$ |
| 16 | Electrical Electrical |  | $\begin{aligned} & 8.60 \\ & 8.60 \end{aligned}$ |  | $\begin{aligned} & 11.43 \\ & 11.43 \end{aligned}$ | $\begin{aligned} & 2,620,616 \\ & 2,620,616 \end{aligned}$ |
| Total Bu | ing Costs |  | 100.00 |  | 132.99 | 30,483,633 |


|  |  | Final Report |  | 2010 |
| :---: | :---: | :---: | :---: | :---: |
| 「uesday, September 29, 2009 |  |  |  | Pag |
| Total Non-Building Costs | 100.00 | 0.00 | 0 |  |
| Total Proiect Costs | -- | -- |  |  |

## Appendix D: RS Means Reference Pages



Costs per square foot of floor area

| Exterior Wall | S.F. Area | 35000 | 55000 | 75000 | 95000 | 115000 | 135000 | 155000 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 314 | 401 | 497 | 555 | 639 | 722 | 754 | 783 |  |
|  | LF. Perimeier |  |  | 169.95 | 166.40 | 164.80 | 163.70 | 161.70 | 160.15 | 159.60 |
| Face Brick with Concrete Block Back-up | Steel Frame | 182.95 | 173.8 | 169.95 |  | 628 | 161.75 | 159.75 | 158.20 | 157.60 |
|  | R/Conc. Frame | 180.95 | 171.85 | 107.95 |  |  |  | 158 | 156.85 | 156.35 |
| Glass and Metal Curtain Walls | Steel Frame | 177.25 | 168.95 | 165.40 | 162.35 | 160.90 | 59.90 | 158. |  |  |
|  | R/Conc. Frame | 175.50 | 167.25 | 163.70 | 160.60 | 159.15 | 158.10 | 156.50 | 155.15 | 154.60 |
|  | R/Conc. Frame | 175.50 |  |  | 170.00 | 168.15 | 166.90 | 164.55 | 162.80 | 162.10 |
| Prẹcast <br> Concrete Panels | Steel Frame | 189.05 | 178.55 | 174.15 | 170.00 |  |  | 163.35 | 161.50 | 160.85 |
|  | R/Conc. Frame | 188.20 | 177.55 | 173.10 | 168.85 | 167.00 | 165.70 | 103.35 | 161.50 | 160.85 |
|  | Per 100 L.F. | 9.50 | 6.05 | 4.40 | 3.50 | 2.85 | 2.45 | 2.15 | 1.95 | 1.75 |
| Perimeter Adj., Add or Deduct |  | 9.50 | 0.05 | 4.40 | 1.70 | 1.60 | 1.60 | 1.45 | 1.30 | 1.25 |
| Story Hgt. Adj., Add or Deduct | Per 1 ft . | 2.65 | 2.1 | 1.95 |  |  |  |  |  |  |

The above costs were calculated using the basic specitications shown on the facing page. These coststre, range from $\$ 108.75$ to $\$ 208.75$ per S.F.
design alternatives and owner's requirements. Reported completed project costs, for this type of structure, range trom $\$ 108.75$ to $\$ 208.75$ per $\$$. F.

## Common additives

Description
Bar, Front bar
Back bar
Booth, Upholstered, custom, straight
" ${ }^{\prime \prime}$ or "U" shaped
Closed Circuit Surveillonce, One station
Camera and monitor
For additional camera stations, add
Direciory Boards, Plastic, glass covered
$30^{\prime \prime} \times 20^{\prime \prime}$
$36^{\prime \prime} \times 48^{\prime \prime}$
Aluminum, $24^{\prime \prime} \times 18^{\prime \prime}$
$48^{\prime \prime} \times 32^{\prime \prime}$
$48^{\prime \prime} \times 60^{\prime \prime}$
Elevators, Electric passenger, 5 stops
$3500 \#$ capacily
$5000 \#$ capacily
Additional stop, add
Emergency Lighting, 25 watt, battery operated
Lead battery
Nickel cadmium

| Unit | \$ Cost | Description | Unit | \$ Cost |
| :---: | :---: | :---: | :---: | :---: |
| L.F. | 360 | Laundry Equipment | Each | 66,500 |
| LF. | 289 | Foiders, blankets \& sheets, king size | Each | 35,500 |
| L.F. | 202.375 | Ironers, $110^{\prime \prime}$ single roll | Each | 12,200 |
| L.F. | $210 \cdot 355$ | Combination washer extractor 50\# 125\# | Each | 32,800 |
| Each | 1850 | Sauna, Prefabricated, complete | Each | 5850 |
| Each | 1000 | $6^{\prime} \times 4^{\prime}$ | Each | 6950 |
|  |  | $6^{\prime} \times 6$ | Each | 8525 |
| Each | 595 | $6^{\prime} \times 9{ }^{\prime}$ | Each | 10,100 |
| Each | 1450 | $8^{\prime} \times 8^{\prime}$ | Each | 14,000 |
| Each | 600 | $10^{\prime} \times 12^{\prime}$ | -ach |  |
| Each | 980 | Smoke Detectors | Each | 187 |
| Each | 2025 | Ceiling type | Each | 480 |
|  |  | Duct type |  |  |
| Each | 167,200 | Sound System | Each | 2350 |
| Each | 170,700 | Ampiriter, 250 watts | Each | 191 |
| Each | 13,600 | Speaker, ceiling or wal | Each | 365 |
|  |  | Trumpet | Outhet | 315 |
| Each | 282 | TV Antenna, Master system, 12 outhet | Outhet | 203 |
| Each | 805 | 30 outhet | Outhet | 194 |


| STATE/ZIP | CITY | Residential | Commercial | STATE/ZIP | CITY | Residential | Commercial |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { UTAH (CONT'd) } \\ & 845 \\ & 846-847 \end{aligned}$ | $\begin{array}{\|l} \text { Price } \\ \text { Provo } \end{array}$ | $\begin{aligned} & .70 \\ & .80 \end{aligned}$ | $\begin{aligned} & .78 \\ & .87 \end{aligned}$ | WYOMING (CONT'd) |  |  |  |
|  |  |  |  | 823 (CONT | Rawlins | . 75 | . 83 |
|  |  |  |  | 824 | Worland | . 74 | . 81 |
|  |  |  | . 80 | 826827 | Riverton Casper | .73 .76 | . 81 |
| 050 | White River Jct. | . 76 |  |  | Newcastle | . 74 | . 81 |
| 051 | Bellows Falls | . 78 | . 82 | 828 | Sheridan | . 79 | . 84 |
| 052 | Bernington Brattleboro | . 80 | . 83 | 829-831 | Rock Springs | . 78 | . 83 |
| 054 | Burlington | . 81 | . 86 | CANADIAN FACTORS (reflect Canadian currency) |  |  |  |
| 056 | Montpelier | . 82 | . 84 |  |  |  |  |
| 057 | Rutland | . 81 | . 85 |  | Calgary |  |  |
| 059 | St. Johnsbury Guildhall | . 78 | . 80 | ALBERTA |  | 1.14 | 1.14 |
|  | Guildahlif | . 77 | . 79 |  | Edmonton Fort McMurra | 1.13 | 1.14 |
| VIRGINIA |  |  |  |  | Fort McMurray Lethbridge | 1.14 1.11 | 1.13 1.09 |
| $220-221$ | Fairfax | 1.02 | . 93 |  | Lloydminster | 1.06 | 1.05 |
| 222 | Arlington | 1.03 | . 93 |  | Medicine Hat | 1.07 | 1.05 |
| $\xrightarrow{2} 23$ | Alexandria Fredericksburg | 1.07 | . 95 |  | Red Deer | 1.07 | 1.05 |
| $\stackrel{24}{2}$ | Fredericksburg Winchester | . 94 | . 88 | BRITISH COLUMBIA |  |  |  |
| 226 | Winchester | . 91 | . 86 |  |  |  |  |
| $\bigcirc 27$ | Culpeper Harrisonburg | .99 .89 | . 88 |  | Kamloops | 1.05 | 1.06 |
| -29 | Harrisonburg Charlottesville | . 89 | . 86 |  | Prince George | 1.05 | 1.07 |
| -30-232 | Richmond | . 98 | . 86 |  | Vancouver | 1.06 99 | 1.11 |
| ?33-235 | Norfolk | 1.00 | . 89 | MANITOBA | Victoria | . 99 | 1.02 |
| $!36$ | Newport News | . 99 | . 88 |  |  |  |  |
| -37 | Portsmouth | . 92 | . 86 |  | Brandon | 1.02 | 1.00 |
| ! 38 | Petersburg Farmville | . 96 | . 87 |  | Portage la Prairie | 1.02 | . 99 |
| !40-241 | Roanoke | . 97 | . 85 |  | Winnipeg | 1.02 | 1.04 |
| $\stackrel{42}{ }$ | Bristol | . 85 | . 81 | NEW BRUNSWICK | Bathurst |  |  |
| 143 | Pulaski | . 83 | . 80 |  |  | . 94 | . 95 |
| 144 | Staunton | . 90 | . 84 |  | Dalhousie | . 94 | . 95 |
| 145 | Grundy | . 95 | . 86 |  | Fredericton | 1.01 | . 98 |
| :46 |  | . 83 | . 80 |  | Moncton | . 95 | . 96 |
|  |  |  |  |  | Newcastle | . 94 | . 95 |
| 80-981,987 | Seattle | 1.02 | 1.04 |  | St. John | 1.01 | . 98 |
| 82 | Everett | 1.04 | 1.02 | NEWFOUNDLAND |  |  |  |
| $85-984$ | Tacoma | 1.02 | 1.03 |  | Corner Brook | . 96 | . 98 |
| 85 | Olympia | 1.01 .97 | 1.02 |  | St. Johns | . 98 | . 99 |
| 88 | Wenatchee | . 92 | . 95 | NORTHWEST TERRITORIES |  |  |  |
| 89 | Yakima | . 96 | . 98 |  | Yellowknife | 1.07 | 1.06 |
| 90.992 | Spokane | . 99 |  |  |  |  |  |
| 94 | Clarkston | . 96 | . 96 |  |  |  |  |
|  |  |  | . 94 | NOVA SCOTIA | BridgewaterDartmouth | . 97 | . 99 |
|  |  |  |  |  |  | . 98 | 1.00 |
| 47-248 | Bluefield |  |  |  | Halifax New Glasgow | 1.00 | 1.02 |
| 49 | Lewisburg | . 88 | .89.92.95 |  | New Glasgow Sydney | . 96 | $\begin{array}{r}.99 \\ \hline 97\end{array}$ |
| 50-253 | Charleston | . 95 |  |  | Truro | . 97 | . 99 |
| 54 | Martinsburg | . 86 | . 90 |  | Yarmouth | . 97 | . 99 |
| 58-259 | Huntington | . 96 | . 96 |  |  |  |  |
| 50 | Beckiey | . 90 | . 93 | ONTARIO |  | 1.13 |  |
| 51 | Wheeiing | . 92 | . 96 |  | Barrie |  | 1.08 |
| 52 | Parkersburg | . 91 | . 95 | Brantford |  | 1.14 | 1.09 |
| j3-264 | Buckhannon | . 91 |  |  | Cornwall | 1.14 |  |
| ${ }_{35}^{33-264}$ | Clarksburg | . 91 | . 95 | Hamiton |  | 1.16 | 1.08 |
| 56 | Morgantown Gassaway | . 92 | . 95 |  | Kingston | 1.14 | 1.12 1.09 |
| 16 7 | Gassaway | . 91 | . 95 | KitchenerLondon |  | 1.09 | 1.05 |
| 88 | Petersburg | . 89 | . 92 |  |  | 1.14 | 1.10 |
|  |  | . 91 | . 93 | North Bay |  | 1.11 | 1.071.08 |
| ISCONSIN |  |  |  | Ottawa |  | 1.13 |  |
| $30,532$ | Milwaukee | 1.07 |  |  |  | 1.16 | 1.11 1.08 |
| 31 | Kenosha | 1.03 | 1.00 | Owen Sound |  | 1.11 | 1.08 |
| 14 | Racine | 1.02 | 1.00 | Sarnia |  | 1.14 | 1.091.04 |
| 15 | Beloit | . 98 | . 97 |  | Sault Ste Marie | 1.07 |  |
| 17 | Madison | . 98 | .98.94 |  | St. Catharines | 1.10 | 1.04 1.05 |
| 18 | Lancaster | . 97 |  |  | Sudbury | 1.07 | 1.04 |
| 19 | Portage | . 96 | . 95 |  | Thunder Bay | 1.12 | 1.05 |
| 0 | New Richmond | . 99 | . 95 |  | Timmins | 1.11 | 1.07 |
| $1-543$ | Green Bay | 1.00 | . 96 |  | Toronto | 1.17 | 1.14 |
| 4 | Wausau | . 94 | . 92 |  | Windsor | 1.11 | 1.05 |
| 5 | Rhinelander | . 94 | . 94 |  |  |  |  |
| 6 | La Crosse | . 94 | . 94 | PRINCE EDWARD ISL | AND |  |  |
| 8 | Eau Claire | . 97 | . 95 |  | Chariottetown | . 92 | 95 |
| 9 | Oshkosh | . 98 | . 96 |  | Summerside | . 92 | . 95 |
|  |  |  |  | QUEBEC |  |  |  |
| 'OMING | Cheyenne <br> Yellowstone Nat. Pk. Wheatland | $\begin{aligned} & .82 \\ & .74 \\ & .74 \\ & \hline \end{aligned}$ | $\begin{aligned} & .86 \\ & .81 \\ & .82 \\ & \hline \end{aligned}$ |  | Cap-dela-Madeleine <br> Charlesbourg <br> Chicoutimi <br> Gatineau | 1.13 <br> 1.13 <br> 1.16 <br> 1.12 | $\begin{aligned} & 1.04 \\ & 1.04 \\ & 1.05 \\ & 1.03 \end{aligned}$ |
| 1 |  |  |  |  |  |  |  |
| 2 |  |  |  |  |  |  |  |

## Appendix E: Detailed Structural Systems Estimate

 Concrete Columns:| Normal Weight Concrete, 3000 psi |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | Size | Quantity | Total CY | Unit Mat'l Cost | Material Cost | Total Cost |
| Area 1 | $18 \times 18$ | 9 | 7.50 | \$ 101.00 | \$ 757.48 | \$ 757.48 |
|  | $24 \times 24$ | 40 | 59.26 | \$ 101.00 | \$5,985.02 | \$ 5,985.02 |
| Area 2 | $18 \times 18$ | 22 | 18.33 | \$ 101.00 | \$ 1,851.61 | \$ 1,851.61 |
|  | $24 \times 24$ | 31 | 45.92 | \$ 101.00 | \$4,638.39 | \$ 4,638.39 |
|  | $24 \times 72$ | 2 | 8.89 | \$ 101.00 | \$ 897.75 | \$ 897.75 |
| Area 3 | $12 \times 12$ | 6 | 2.22 | \$ 101.00 | \$ 224.44 | \$ 224.44 |
|  | $12 \times 16$ | 2 | 0.99 | \$ 101.00 | \$ 99.75 | \$ 99.75 |
|  | 18×18 | 21 | 17.50 | \$ 101.00 | \$1,767.45 | \$ 1,767.45 |
|  | 18× 36 | 5 | 8.33 | \$ 101.00 | \$ 841.64 | \$ 841.64 |
|  | 24×24 | 7 | 10.37 | \$ 101.00 | \$ 1,047.38 | \$ 1,047.38 |
|  | $26 \times 26$ | 5 | 8.69 | \$ 101.00 | \$ 878.01 | \$ 878.01 |
| Area 4 | $10 \times 30$ | 4 | 3.09 | \$ 101.00 | \$ 311.72 | \$ 311.72 |
|  | $12 \times 12$ | 11 | 4.07 | \$ 101.00 | \$ 411.47 | \$ 411.47 |
|  | 12× 24 | 8 | 5.93 | \$ 101.00 | \$ 598.50 | \$ 598.50 |
|  | 16×24 | 2 | 1.98 | \$ 101.00 | \$ 199.50 | \$ 199.50 |
|  | $16 \times 28$ | 73 | 84.11 | \$ 101.00 | \$8,495.40 | \$ 8,495.40 |
|  |  |  |  |  | Total | \$29,005.51 |


| Placing Concrete, pumped |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | Size | Quantity | Total CY | Unit Labor Cost |  | Labor Cost | Unit Equipment |  | Equipment Cost | Total Cost |
| Area 1 | $18 \times 18$ | 9 | 7.50 | \$ | 24.00 | \$ 179.99 | \$ | 8.80 | \$ 66.00 | \$ 245.99 |
|  | $24 \times 24$ | 40 | 59.26 | \$ | 23.50 | \$ 1,392.55 | \$ | 8.60 | \$ 509.62 | \$ 1,902.17 |
| Area 2 | $18 \times 18$ | 22 | 18.33 | \$ | 24.00 | \$ 439.99 | \$ | 8.80 | \$ 161.33 | \$ 601.32 |
|  | 24× 24 | 31 | 45.92 | \$ | 23.50 | \$ 1,079.23 | \$ | 8.60 | \$ 394.95 | \$ 1,474.18 |
|  | $24 \times 72$ | 2 | 8.89 | \$ | 15.50 | \$ 137.77 | \$ | 5.65 | \$ 50.22 | \$ 187.99 |
| Area 3 | $12 \times 12$ | 6 | 2.22 | \$ | 36.00 | \$ 80.00 | \$ | 13.15 | \$ 29.22 | \$ 109.22 |
|  | $12 \times 16$ | 2 | 0.99 | \$ | 24.00 | \$ 23.70 | \$ | 8.80 | \$ 8.69 | \$ 32.39 |
|  | $18 \times 18$ | 21 | 17.50 | \$ | 24.00 | \$ 419.99 | \$ | 8.80 | \$ 154.00 | \$ 573.98 |
|  | $18 \times 36$ | 5 | 8.33 | \$ | 15.50 | \$ 129.16 | \$ | 5.65 | \$ 47.08 | \$ 176.25 |
|  | $24 \times 24$ | 7 | 10.37 | \$ | 23.50 | \$ 243.70 | \$ | 8.60 | \$ 89.18 | \$ 332.88 |
|  | $26 \times 26$ | 5 | 8.69 | \$ | 15.50 | \$ 134.74 | \$ | 5.65 | \$ 49.12 | \$ 183.86 |
| Area 4 | $10 \times 30$ | 4 | 3.09 | \$ | 23.50 | \$ 72.53 | \$ | 8.60 | \$ 26.54 | \$ 99.07 |
|  | $12 \times 12$ | 11 | 4.07 | \$ | 36.00 | \$ 146.66 | \$ | 13.15 | \$ 53.57 | \$ 200.24 |
|  | 12× 24 | 8 | 5.93 | \$ | 23.50 | \$ 139.26 | \$ | 8.60 | \$ 50.96 | \$ 190.22 |
|  | $16 \times 24$ | 2 | 1.98 | \$ | 23.50 | \$ 46.42 | \$ | 8.60 | \$ 16.99 | \$ 63.41 |
|  | $16 \times 28$ | 73 | 84.11 | \$ | 15.50 | \$ 1,303.75 | \$ | 5.65 | \$ 475.24 | \$ 1,778.99 |
|  |  |  |  |  |  |  |  |  | Total | \$8,152.15 |


| Forms in Place, plywood 2 use |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | Size | Quantity | SFCA | Unit Mat'l Cost |  | Material Cost |  | Unit Labor Cost |  | Labor Cost | Total Cost |  |
| Area 1 | $18 \times 18$ | 9 | 26.00 | \$ | 1.37 | \$ | 35.62 | \$ | 5.60 | \$ 145.60 | \$ | 181.22 |
|  | $24 \times 24$ | 40 | 28.00 | \$ | 1.37 | \$ | 38.36 | \$ | 5.60 | \$ 156.80 | \$ | 195.16 |
| Area 2 | 18× 18 | 22 | 26.00 | \$ | 1.37 | \$ | 35.62 | \$ | 5.60 | \$ 145.60 | \$ | 181.22 |
|  | 24× 24 | 31 | 28.00 | \$ | 1.37 | \$ | 38.36 | \$ | 5.60 | \$ 156.80 | \$ | 195.16 |
|  | 24×72 | 2 | 36.00 | \$ | 1.03 | \$ | 37.08 | \$ | 6.28 | \$ 226.08 | \$ | 263.16 |
| Area 3 | $12 \times 12$ | 6 | 24.00 | \$ | 1.25 | \$ | 30.00 | \$ | 5.75 | \$ 138.00 | \$ | 168.00 |
|  | $12 \times 16$ | 2 | 24.67 | \$ | 1.19 | \$ | 29.35 | \$ | 5.65 | \$ 139.37 |  | 168.72 |
|  | $18 \times 18$ | 21 | 26.00 | \$ | 1.37 | \$ | 35.62 | \$ | 5.60 | \$ 145.60 |  | 181.22 |
|  | $18 \times 36$ | 5 | 29.00 | \$ | 1.03 | \$ | 29.87 | \$ | 6.28 | \$ 182.12 |  | 211.99 |
|  | 24× 24 | 7 | 28.00 | \$ | 1.37 | \$ | 38.36 | \$ | 5.60 | \$ 156.80 |  | 195.16 |
|  | $26 \times 26$ | 5 | 28.67 | \$ | 1.03 | \$ | 29.53 | \$ | 6.28 | \$ 180.03 | \$ | 209.55 |
| Area 4 | $10 \times 30$ | 4 | 26.67 | \$ | 1.03 | \$ | 27.47 | \$ | 6.28 | \$ 167.47 | \$ | 194.93 |
|  | 12x 12 | 11 | 24.00 | \$ | 1.25 | \$ | 30.00 | \$ | 5.75 | \$ 138.00 | \$ | 168.00 |
|  | 12× 24 | 8 | 26.00 | \$ | 1.37 | \$ | 35.62 | \$ | 5.60 | \$ 145.60 | \$ | 181.22 |
|  | 16x 24 | 2 | 26.67 | \$ | 1.37 | \$ | 36.53 | \$ | 5.60 | \$ 149.33 | \$ | 185.87 |
|  | $16 \times 28$ | 73 | 27.33 | \$ | 1.03 | \$ | 28.15 | \$ | 6.28 | \$ 171.65 | \$ | 199.81 |
|  |  |  |  |  |  |  |  |  |  | Total |  | 080.39 |

## Concrete Slabs:

| Normal Weight Concrete |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | Type of Slab | Area (SF) | Depth (in) | PSI | Total CY |  | nit Mat'l Cost | Material Cost | Total Cost |
| Area 1 | SOG | 20500 | 5 | 3000 | 43.94 | \$ | 101.00 | \$ 4,437.80 | \$ 4,437.80 |
| Area 2 | SOG | 20400 | 5 | 3000 | 43.72 | \$ | 101.00 | \$ 4,416.15 | \$ 4,416.15 |
| Area 3 | SOG | 17100 | 5 | 3000 | 36.65 | \$ | 101.00 | \$ 3,701.77 | \$ 3,701.77 |
| Area 4.B | SOG | 30000 | 5 | 3000 | 64.30 | \$ | 104.00 | \$ 6,687.24 | \$ 6,687.24 |
| Area 4.1 | Elevated | 30000 | 9 | 3500 | 208.33 | \$ | 104.00 | \$21,666.64 | \$ 21,666.64 |
| Area 4.2 | Elevated | 30000 | 9 | 3500 | 208.33 | \$ | 104.00 | \$21,666.64 | \$ 21,666.64 |
| Area 4.3 | Elevated | 30000 | 9 | 3500 | 208.33 | \$ | 104.00 | \$21,666.64 | \$ 21,666.64 |
| Area 4.P | Elevated | 30000 | 10 | 3500 | 257.20 | \$ | 104.00 | \$26,748.94 | \$ 26,748.94 |
|  |  |  |  |  |  |  |  | Total | \$110,991.83 |


| Placing, pumped |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | Type of Slab | Depth (in) | Total CY | Unit Labor Cost | Labor Cost |  | it <br> ment | Equipment Cost |  | otal Cost |
| Area 1 | SOG | 5 | 43.94 | 16.7 | 733.77 | \$ | 6.10 | \$ 268.03 | \$ | 1,001.80 |
| Area 2 | SOG | 5 | 43.72 | 16.7 | 730.19 | \$ | 6.10 | \$ 266.72 | \$ | 996.91 |
| Area 3 | SOG | 5 | 36.65 | 16.7 | 612.08 | \$ | 6.10 | \$ 223.57 | \$ | 835.65 |
| Area 4.B | SOG | 5 | 64.30 | 16.7 | 1073.82 | \$ | 6.10 | \$ 392.23 | \$ | 1,466.05 |
| Area 4.1 | Elevated | 9 | 208.33 | 13.55 | 2822.91 | \$ | 4.94 | \$ 1,029.17 | \$ | 3,852.08 |
| Area 4.2 | Elevated | 9 | 208.33 | 13.55 | 2822.91 | \$ | 4.94 | \$ 1,029.17 | \$ | 3,852.08 |
| Area 4.3 | Elevated | 9 | 208.33 | 13.55 | 2822.91 | \$ | 4.94 | \$ 1,029.17 | \$ | 3,852.08 |
| Area 4.P | Elevated | 10 | 257.20 | 13.55 | 3485.08 | \$ | 4.94 | \$ 1,270.57 | \$ | 4,755.65 |
|  |  |  |  |  |  |  |  | Total | \$ | 20,612.30 |


| Forming |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | Type of Slab | SFCA | Depth (in) | L.F. | Unit Mat'I Cost |  | Material Cost |  | Unit Labor Cost |  | Labor Cost |  | Total Cost |
| Area 1 | SOG | 295.83 | 5 | 710.00 | \$ | 0.46 | \$ | 326.60 | \$ | 3.03 | \$ | 2,151.30 | \$ 2,477.90 |
| Area 2 | SOG | 254.17 | 5 | 610.00 | \$ | 0.46 | \$ | 280.60 | \$ | 3.03 | \$ | 1,848.30 | \$ 2,128.90 |
| Area 3 | SOG | 291.67 | 5 | 700.00 | \$ | 0.46 | \$ | 322.00 | \$ | 3.03 | \$ | 2,121.00 | \$ 2,443.00 |
| Area 4.B | SOG | 500.00 | 5 | 1200.00 | \$ | 0.46 | \$ | 552.00 | \$ | 3.03 | \$ | 3,636.00 | \$ 4,188.00 |
| Area 4.1 | Elevated | 900.00 | 9 | 1200.00 | \$ | 0.70 | \$ | 840.00 | \$ | 5.90 | \$ | 7,080.00 | \$ 7,920.00 |
| Area 4.2 | Elevated | 900.00 | 9 | 1200.00 | \$ | 0.70 | \$ | 840.00 | \$ | 5.90 | \$ | 7,080.00 | \$ 7,920.00 |
| Area 4.3 | Elevated | 900.00 | 9 | 1200.00 | \$ | 0.70 | \$ | 840.00 | \$ | 5.90 | \$ | 7,080.00 | \$ 7,920.00 |
| Area 4.P | Elevated | 1000.00 | 10 | 1200.00 | \$ | 0.70 | \$ | 840.00 | \$ | 5.90 | \$ | 7,080.00 | \$ 7,920.00 |
|  |  |  |  |  |  |  |  |  |  |  | Tot |  | \$42,917.80 |

Footings

| Normal Weight Concrete, 3000 psi |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Width <br> (ft.) | Length <br> (ft.) | Depth <br> (in.) | Quantity | Total CY | Unit Mat'I Cost | Material Cost | Total Cost |
| 4 | 4 | 12 | 4 | 2.37 | \$ 101.00 | \$ 239.41 | \$ 239.41 |
| 4.5 | 4.5 | 12 | 9 | 6.75 | \$ 101.00 | \$ 681.75 | \$ 681.75 |
| 5 | 5 | 12 | 8 | 7.41 | \$ 101.00 | \$ 748.15 | \$ 748.15 |
| 5.5 | 5.5 | 13 | 17 | 20.63 | \$ 101.00 | \$ 2,083.98 | \$ 2,083.98 |
| 6 | 6 | 14 | 42 | 65.33 | \$ 101.00 | \$ 6,598.66 | \$ 6,598.66 |
| 6.5 | 6.5 | 16 | 17 | 35.47 | \$ 101.00 | \$ 3,582.38 | \$ 3,582.38 |
| 7 | 7 | 17 | 24 | 61.70 | \$ 101.00 | \$ 6,232.07 | \$ 6,232.07 |
| 7.5 | 7.5 | 18 | 31 | 96.87 | \$ 101.00 | \$ 9,784.37 | \$ 9,784.37 |
| 8 | 8 | 19 | 10 | 37.53 | \$ 101.00 | \$ 3,790.61 | \$ 3,790.61 |
| 8.5 | 8.5 | 20 | 11 | 49.06 | \$ 101.00 | \$ 4,954.92 | \$ 4,954.92 |
| 9 | 9 | 21 | 2 | 10.50 | \$ 101.00 | \$ 1,060.50 | \$ 1,060.50 |
| 9.5 | 9.5 | 22 | 10 | 61.28 | \$ 101.00 | \$ 6,189.36 | \$ 6,189.36 |
| 10.5 | 10.5 | 25 | 12 | 102.08 | \$ 101.00 | \$10,310.41 | \$ 10,310.41 |
| 12 | 12 | 28 | 1 | 12.44 | \$ 101.00 | \$ 1,256.89 | \$ 1,256.89 |
| 12.5 | 12.6 | 28 | 12 | 163.33 | \$ 101.00 | \$16,496.65 | \$ 16,496.65 |
| 13 | 13 | 30 | 12 | 187.78 | \$ 101.00 | \$18,965.54 | \$ 18,965.54 |
| 14 | 14 | 31 | 10 | 187.53 | \$ 101.00 | \$18,940.60 | \$ 18,940.60 |
| 15 | 15 | 34 | 1 | 23.61 | \$ 101.00 | \$ 2,384.72 | \$ 2,384.72 |
| 9 | 12 | 19 | 2 | 12.67 | \$ 101.00 | \$ 1,279.33 | \$ 1,279.33 |
| 10 | 14 | 26 | 1 | 11.23 | \$ 101.00 | \$ 1,134.69 | \$ 1,134.69 |
| 10 | 15 | 24 | 3 | 33.33 | \$ 101.00 | \$ 3,366.66 | \$ 3,366.66 |
| 18.5 | 28 | 24 | 1 | 38.37 | \$ 101.00 | \$ 3,875.40 | \$ 3,875.40 |
| 5 | 8 | 16 | 1 | 1.98 | \$ 101.00 | \$ 199.51 | \$ 199.51 |
|  |  |  |  |  |  | Total | \$124,156.54 |


| Placing Concrete Footings, Pumped |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Width (ft.) | Length (ft.) | Depth (in.) | Quantity | Total CY | Unit Labor Cost | Labor Cost | UnitEquipment |  | Equipment Cost | Total Cost |
| 4 | 4 | 12 | 4 | 2.37 | \$ 14.45 | 34.25 | \$ | 5.25 | \$ 12.44 | \$ 46.70 |
| 4.5 | 4.5 | 12 | 9 | 6.75 | \$ 14.45 | 97.54 | \$ | 5.25 | \$ 35.44 | \$ 132.97 |
| 5 | 5 | 12 | 8 | 7.41 | 14.45 | 107.04 | \$ | 5.25 | 38.89 | 145.93 |
| 5.5 | 5.5 | 13 | 17 | 20.63 | \$ 14.45 | \$ 298.15 | \$ | 5.25 | 108.33 | 406.48 |
| 6 | 6 | 14 | 42 | 65.33 | 14.45 | 944.07 | \$ | 5.25 | \$ 343.00 | 1,287.07 |
| 6.5 | 6.5 | 16 | 17 | 35.47 | \$ 14.45 | \$ 512.53 | \$ | 5.25 | \$ 186.21 | \$ 698.74 |
| 7 | 7 | 17 | 24 | 61.70 | \$ 14.45 | \$ 891.62 | \$ | 5.25 | \$ 323.94 | \$ 1,215.56 |
| 7.5 | 7.5 | 18 | 31 | 96.87 | \$ 14.45 | \$ 1,399.84 | \$ | 5.25 | \$ 508.59 | \$ 1,908.44 |
| 8 | 8 | 19 | 10 | 37.53 | \$ 14.45 | 542.32 | \$ | 5.25 | \$ 197.04 | \$ 739.36 |
| 8.5 | 8.5 | 20 | 11 | 49.06 | \$ 14.45 | \$ 708.90 | \$ | 5.25 | \$ 257.56 | 966.45 |
| 9 | 9 | 21 | 2 | 10.50 | \$ 14.45 | 151.72 | \$ | 5.25 | 55.12 | 206.85 |
| 9.5 | 9.5 | 22 | 10 | 61.28 | \$ 14.45 | 885.51 | \$ | 5.25 | \$ 321.72 | \$ 1,207.23 |
| 10.5 | 10.5 | 25 | 12 | 102.08 | 14.45 | \$ 1,475.10 | \$ | 5.25 | 535.94 | \$ 2,011.04 |
| 12 | 12 | 28 | 1 | 12.44 | 14.45 | 179.82 | \$ | 5.25 | 65.33 | 245.16 |
| 12.5 | 12.6 | 28 | 12 | 163.33 | 14.45 | \$ 2,360.16 | \$ | 5.25 | 857.50 | \$ 3,217.66 |
| 13 | 13 | 30 | 12 | 187.78 | \$ 14.45 | \$ 2,713.39 | \$ | 5.25 | \$ 985.83 | \$ 3,699.22 |
| 14 | 14 | 31 | 10 | 187.53 | 14.45 | \$ 2,709.82 | \$ | 5.25 | \$ 984.54 | \$ 3,694.35 |
| 15 | 15 | 34 | 1 | 23.61 | \$ 14.45 | 341.18 | \$ | 5.25 | 123.96 | 465.14 |
| 9 | 12 | 19 | 2 | 12.67 | \$ 14.45 | \$ 183.03 | \$ | 5.25 | 66.50 | 249.53 |
| 10 | 14 | 26 | 1 | 11.23 | \$ 14.45 | 162.34 | \$ | 5.25 | 58.98 | 221.32 |
| 10 | 15 | 24 | 3 | 33.33 | \$ 14.45 | \$ 481.67 | \$ | 5.25 | \$ 175.00 | 656.67 |
| 18.5 | 28 | 24 | 1 | 38.37 | \$ 14.45 | 554.45 | \$ | 5.25 | \$ 201.44 | \$ 755.90 |
| 5 | 8 | 16 | 1 | 1.98 | \$ 14.45 | \$ 28.54 | \$ | 5.25 | \$ 10.37 | \$ 38.91 |
|  |  |  |  |  |  |  |  |  | Total | \$24,216.67 |


| Forms in Place, Plywood, 2 use |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Width (ft.) | Length (ft.) | Depth <br> (in.) | Quantity | SFCA |  | nit Mat'\| Cost |  | Material Cost |  | Unit Labor Cost | Labor Cost | Total Cost |
| 4 | 4 | 12 | 4 | 72.00 | \$ | 4.10 | \$ | 295.20 | \$ | 2.75 | 198.00 | \$ 493.20 |
| 4.5 | 4.5 | 12 | 9 | 180.00 | \$ | 4.10 | \$ | 738.00 | \$ | 2.75 | \$ 495.00 | \$ 1,233.00 |
| 5 | 5 | 12 | 8 | 176.00 | + | 4.10 | \$ | 721.60 | \$ | 2.75 | \$ 484.00 | \$ 1,205.60 |
| 5.5 | 5.5 | 13 | 17 | 410.83 | \$ | 4.10 | \$ | 1,684.42 | \$ | 2.75 | \$1,129.79 | \$ 2,814.21 |
| 6 | 6 | 14 | 42 | 1106.00 | \$ | 4.10 | \$ | 4,534.60 | \$ | 2.75 | \$3,041.50 | \$ 7,576.10 |
| 6.5 | 6.5 | 16 | 17 | 487.33 | \$ | 4.10 | \$ | 1,998.07 | \$ | 2.75 | \$1,340.17 | \$ 3,338.23 |
| 7 | 7 | 17 | 24 | 740.00 | \$ | 4.10 | \$ | 3,034.00 | \$ | 2.75 | \$2,035.00 | \$ 5,069.00 |
| 7.5 | 7.5 | 18 | 31 | 1023.00 | \$ | 4.10 | \$ | 4,194.30 | \$ | 2.75 | \$2,813.25 | \$ 7,007.55 |
| 8 | 8 | 19 | 10 | 351.67 | \$ | 4.10 | \$ | 1,441.83 | \$ | 2.75 | \$ 967.08 | \$ 2,408.92 |
| 8.5 | 8.5 | 20 | 11 | 410.67 | \$ | 4.10 |  | 1,683.73 | \$ | 2.75 | \$1,129.33 | \$ 2,813.07 |
| 9 | 9 | 21 |  | 79.00 | \$ | 4.10 | \$ | 323.90 | \$ | 2.75 | \$ 217.25 | \$ 541.15 |
| 9.5 | 9.5 | 22 | 10 | 416.67 | \$ | 4.10 | \$ | 1,708.33 | \$ | 2.75 | \$1,145.83 | \$ 2,854.17 |
| 10.5 | 10.5 | 25 | 12 | 554.00 | \$ | 4.10 | \$ | 2,271.40 | \$ | 2.75 | \$1,523.50 | \$ 3,794.90 |
| 12 | 12 | 28 | 1 | 52.67 | \$ | 4.10 | \$ | 215.93 | \$ | 2.75 | \$ 144.83 | \$ 360.77 |
| 12.5 | 12.6 | 28 | 12 | 658.40 | \$ | 4.10 | \$ | 2,699.44 | \$ | \$ 2.75 | \$1,810.60 | \$ 4,510.04 |
| 13 | 13 | 30 | 12 | 684.00 | \$ | 4.10 | \$ | 2,804.40 | \$ | + 2.75 | \$1,881.00 | \$ 4,685.40 |
| 14 | 14 | 31 | 10 | 611.67 | \$ | 4.10 | \$ | 2,507.83 | \$ | 2.75 | \$1,682.08 | \$ 4,189.92 |
| 15 | 15 | 34 | 1 | 65.67 | \$ | 4.10 | \$ | 269.23 | \$ | + 2.75 | \$ 180.58 | \$ 449.82 |
| 9 | 12 | 19 | 2 | 90.33 | \$ | 4.10 | \$ | 370.37 | \$ | 2.75 | \$ 248.42 | \$ 618.78 |
| 10 | 14 | 26 | 1 | 52.33 | \$ | 4.10 | \$ | 214.57 | \$ | 2.75 | \$ 143.92 | \$ 358.48 |
| 10 | 15 | 24 | 3 | 162.00 | \$ | 4.10 | \$ | 664.20 | \$ | 2.75 | \$ 445.50 | \$ 1,109.70 |
| 18.5 | 28 | 24 | 1 | 97.00 | \$ | 4.10 | \$ | 397.70 | \$ | \$ 2.75 | \$ 266.75 | \$ 664.45 |
| 5 | 8 | 16 | 1 | 28.67 | \$ | 4.10 | \$ | 117.53 | \$ | 2.75 | \$ 78.83 | \$ 196.37 |
|  |  |  |  |  |  |  |  |  |  |  | Total | \$58,292.82 |

Concrete Beams:

| Normal Weight Concrete 3000 psi |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Size |  |  | Length Range (ft.) | $\begin{array}{\|c\|} \hline \text { Avg. Length } \\ (\mathrm{ft} .) \end{array}$ | Quantity | Total CY | Unit Mat'I Cost | Material Cost | Total Cost |
| $12 \times 24$ |  |  |  |  |  |  |  |  |  |
|  |  |  | 10.15 | 12.5 | 1 | 0.93 | \$ 101.00 | \$ 93.52 | \$ 93.52 |
| 16 | $\times 2$ | 24 |  |  |  |  |  |  |  |
|  |  |  | 5.10 | 7.5 | 1 | 0.74 | \$ 101.00 | \$ 74.81 | \$ 74.81 |
|  |  |  | 10.15 | 12.5 | 2 | 2.47 | \$ 101.00 | \$ 249.38 | \$ 249.38 |
| $18 \times 22$ |  |  |  |  |  |  |  |  |  |
|  |  |  | 0.5 | 7.5 | 1 | 0.76 | \$ 101.00 | \$ 77.15 | \$ 77.15 |
| 18 | $\times 3$ | 32 |  |  |  |  |  |  |  |
|  |  |  | 10.15 | 12.5 | 1 | 1.85 | \$ 101.00 | \$ 187.04 | \$ 187.04 |
| $22 \times 24$ |  |  |  |  |  |  |  |  |  |
|  |  |  | 5.10 | 7.5 | 1 | 1.02 | \$ 101.00 | \$ 102.87 | \$ 102.87 |
| 24 | $\times 2$ | 24 |  |  |  |  |  |  |  |
|  |  |  | O. 5 | 7.5 | 2 | 2.22 | \$ 101.00 | \$ 224.44 | \$ 224.44 |
|  |  |  | 5.10 | 7.5 | 2 | 2.22 | \$ 101.00 | \$ 224.44 | \$ 224.44 |
|  |  |  | 10.15 | 12.5 | 2 | 3.70 | \$ 101.00 | \$ 374.07 | \$ 374.07 |
|  |  |  | 15.20 | 17.5 | 3 | 7.78 | \$ 101.00 | \$ 785.55 | \$ 785.55 |
|  |  |  | 20.25 | 22.5 | 10 | 33.33 | \$ 101.00 | \$3,366.66 | \$ 3,366.66 |
|  |  |  | 25.30 | 27.5 | 5 | 20.37 | \$ 101.00 | \$2,057.41 | \$ 2,057.41 |
| $24 \times 30$ |  |  |  |  |  |  |  |  |  |
|  |  |  | 10.15 | 12.5 | 1 | 2.31 | \$ 101.00 | \$ 233.80 | \$ 233.80 |
|  |  |  | 15.20 | 17.5 | 1 | 3.24 | \$ 101.00 | \$ 327.31 | \$ 327.31 |
|  |  |  | 20.25 | 22.5 | 4 | 16.67 | \$ 101.00 | \$1,683.33 | \$ 1,683.33 |
|  |  |  | 25.30 | 27.5 | 2 | 10.19 | \$ 101.00 | \$1,028.70 | \$ 1,028.70 |
| $24 \times 32$ |  |  |  |  |  |  |  |  |  |
|  |  |  | 10.15 | 12.5 | 1 | 2.47 | \$ 101.00 | \$ 249.38 | \$ 249.38 |
| $24 \times 57$ ( |  |  |  |  |  |  |  |  |  |
|  |  |  | 20.25 | 22.5 | 1 | 7.92 | \$ 101.00 | \$ 799.58 | \$ 799.58 |
|  |  |  | 25.30 | 27.5 | 1 | 9.68 | \$ 101.00 | \$ 977.27 | \$ 977.27 |
| $12 \times 18$ |  |  |  |  |  |  |  |  |  |
|  |  |  | 5.10 | 7.5 | 4 | 1.67 | \$ 101.00 | \$ 168.33 | \$ 168.33 |
| $12 \times 24$ |  |  |  |  |  |  |  |  |  |
|  |  |  | 10. 15 | 12.5 | 13 | 12.04 | \$ 101.00 | \$1,215.74 | \$ 1,215.74 |
|  |  |  | 15.20 | 17.5 | 6 | 7.78 | \$ 101.00 | \$ 785.55 | \$ 785.55 |
|  |  |  | 20.25 | 22.5 | 6 | 10.00 | \$ 101.00 | \$1,010.00 | \$ 1,010.00 |
| $12 \times 36$ |  |  |  |  |  |  |  |  |  |
|  |  |  | 10.15 | 12.5 | 4 | 5.56 | \$ 101.00 | \$ 561.11 | \$ 561.11 |
|  |  |  | 15.20 | 17.5 | 2 | 3.89 | \$ 101.00 | \$ 392.78 | \$ 392.78 |
|  |  |  | 15.20 | 17.5 | 2 | 3.89 | \$ 101.00 | \$ 392.78 | \$ 392.78 |
| $12 \times 38$ |  |  |  |  |  |  |  |  |  |
|  |  |  | 10.15 | 12.5 | 1 | 1.47 | \$ 101.00 | \$ 148.07 | \$ 148.07 |
|  |  |  |  |  |  |  |  |  |  |
| Post-Tensioning Beams |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| $18 \times 22$ |  |  |  |  |  |  |  |  |  |
|  |  |  |  | 19 | 8 | 15.48 | \$ 101.00 | \$1,563.63 | \$ 1,563.63 |
|  |  |  |  | 30 | 2 | 6.11 | \$ 101.00 | \$ 617.22 | \$ 617.22 |
|  |  |  |  | 40 | 8 | 32.59 | \$ 101.00 | \$3,291.85 | \$ 3,291.85 |
|  |  |  |  | 50 | 9 | 45.83 | \$ 101.00 | \$4,629.16 | \$ 4,629.16 |
|  |  |  |  |  |  |  |  |  |  |
|  |  |  |  | 50 | 1 | 5.79 | \$ 101.00 | \$ 584.49 | \$ 584.49 |
|  |  |  |  |  |  |  |  |  |  |
|  |  |  |  | 30 | 1 | 4.07 | \$ 101.00 | \$ 411.48 | \$ 411.48 |
| $22 \times 32$ |  |  |  | 40 | 1 | 5.43 | \$ 101.00 | \$ 548.64 | \$ 548.64 |
|  |  |  |  |  |  |  |  |  |  |
|  |  |  |  | 19 | 2 | 6.88 | \$ 101.00 | \$ 694.95 | \$ 694.95 |
|  |  |  |  | 40 | 2 | 14.49 | \$ 101.00 | \$1,463.04 | \$ 1,463.04 |
|  |  |  |  | 50 | 1 | 9.05 | \$ 101.00 | \$ 914.40 | \$ 914.40 |
| $22 \times 36$ |  |  |  |  |  |  |  |  |  |
|  |  |  |  | 19 | 1 | 3.87 | \$ 101.00 | \$ 390.91 | \$ 390.91 |
|  |  |  |  | 40 | 1 | 8.15 | \$ 101.00 | \$ 822.96 | \$ 822.96 |
|  |  |  |  | 50 | 1 | 10.19 | \$ 101.00 | \$1,028.70 | \$ 1,028.70 |
| $24 \times 36$ |  |  |  |  |  |  |  |  |  |
|  |  |  | 10.15 | 12.5 | 1 | 2.78 | \$ 101.00 | \$ 280.56 | \$ 280.56 |
|  |  |  | 20. 25 | 22.5 | 2 | 10.00 | \$ 101.00 | \$1,010.00 | \$ 1,010.00 |
|  |  |  | 25.30 | 27.5 | 2 | 12.22 | \$ 101.00 | \$1,234.44 | \$ 1,234.44 |
|  |  |  |  |  |  |  |  | Total | \$36,043.09 |


| Placing Concrete, Pumped |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Size |  | Length Range (ft.) | Avg. Length (ft.) | Quantity | Total CY | Unit Labor Cost | Labor Cost | Unit Equipment Cost | Equipment Cost |  | otal Cost |
| 12 | $\times 24$ |  |  |  |  |  |  |  |  |  |  |
|  |  | 10.15 | 12.5 | 1 | 0.93 | \$36.00 | \$ 33.33 | \$ 13.15 | \$ 12.18 | \$ | 45.51 |
| $16 \times 24$ |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 5.10 | 7.5 | 1 | 0.74 | \$36.00 | \$ 26.67 | \$ 13.15 | \$ 9.74 | \$ | 36.41 |
|  |  | 10.15 | 12.5 | 2 | 2.47 | \$36.00 | \$ 88.89 | \$ 13.15 | \$ 32.47 | \$ | 121.36 |
| $18 \times 22$ |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 0.5 | 7.5 | 1 | 0.76 | \$36.00 | \$ 27.50 | \$ 13.15 | \$ 10.05 | \$ | 37.55 |
| $18 \times 32$ |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 10.15 | 12.5 | 1 | 1.85 | \$36.00 | \$ 66.67 | \$ 13.15 | \$ 24.35 | \$ | 91.02 |
| $22 \times 24$ |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 5.10 | 7.5 | 1 | 1.02 | \$36.00 | \$ 36.67 | \$ 13.15 | \$ 13.39 | \$ | 50.06 |
| 24×24 |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 0.5 | 7.5 | 2 | 2.22 | \$36.00 | \$ 80.00 | \$ 13.15 | \$ 29.22 | \$ | 109.22 |
|  |  | 5.10 | 7.5 | 2 | 2.22 | \$36.00 | \$ 80.00 | \$ 13.15 | \$ 29.22 | \$ | 109.22 |
|  |  | 10.15 | 12.5 | 2 | 3.70 | \$36.00 | \$ 133.33 | \$ 13.15 | \$ 48.70 | \$ | 182.04 |
|  |  | 15.20 | 17.5 | 3 | 7.78 | \$36.00 | \$ 280.00 | \$ 13.15 | \$ 102.28 | \$ | 382.28 |
|  |  | 20.25 | 22.5 | 10 | 33.33 | \$36.00 | \$1,200.00 | \$ 13.15 | \$ 438.33 | \$ | 1,638.33 |
|  |  | 25.30 | 27.5 | 5 | 20.37 | \$36.00 | \$ 733.33 | \$ 13.15 | \$ 267.87 | \$ | 1,001.20 |
| $24 \times 30$ |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 10.15 | 12.5 | 1 | 2.31 | \$36.00 | \$ 83.33 | \$ 13.15 | \$ 30.44 | \$ | 113.77 |
|  |  | 15.20 | 17.5 | 1 | 3.24 | \$36.00 | \$ 116.67 | \$ 13.15 | \$ 42.62 | \$ | 159.28 |
|  |  | 20.25 | 22.5 | 4 | 16.67 | \$36.00 | \$ 600.00 | \$ 13.15 | \$ 219.17 | \$ | 819.17 |
|  |  | 25.30 | 27.5 | 2 | 10.19 | \$36.00 | \$ 366.67 | \$ 13.15 | \$ 133.94 | \$ | 500.60 |
| $24 \times 32$ |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 10.15 | 12.5 | 1 | 2.47 | \$36.00 | \$ 88.89 | \$ 13.15 | \$ 32.47 | \$ | 121.36 |
| $24 \times 57$ |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 20.25 | 22.5 | 1 | 7.92 | \$36.00 | \$ 285.00 | \$ 13.15 | \$ 104.10 | \$ | 389.10 |
|  |  | 25.30 | 27.5 | 1 | 9.68 | \$36.00 | \$ 348.33 | \$ 13.15 | \$ 127.24 | \$ | 475.57 |
| $12 \times 18$ |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 5.10 | 7.5 | 4 | 1.67 | \$36.00 | \$ 60.00 | \$ 13.15 | \$ 21.92 | \$ | 81.92 |
| $12 \times 24$ |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 10.15 | 12.5 | 13 | 12.04 | \$36.00 | \$ 433.33 | \$ 13.15 | \$ 158.29 | \$ | 591.62 |
|  |  | 15.20 | 17.5 | 6 | 7.78 | \$36.00 | \$ 280.00 | \$ 13.15 | \$ 102.28 | \$ | 382.28 |
|  |  | 20.25 | 22.5 | 6 | 10.00 | \$36.00 | \$ 360.00 | \$ 13.15 | \$ 131.50 | \$ | 491.50 |
| $12 \times 36$ |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 10.15 | 12.5 | 4 | 5.56 | \$36.00 | \$ 200.00 | \$ 13.15 | \$ 73.06 | \$ | 273.06 |
|  |  | 15.20 | 17.5 | 2 | 3.89 | \$36.00 | \$ 140.00 | \$ 13.15 | \$ 51.14 | \$ | 191.14 |
|  |  | 15.20 | 17.5 | 2 | 3.89 | \$36.00 | \$ 140.00 | \$ 13.15 | \$ 51.14 | \$ | 191.14 |
| $12 \times 38$ |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 10.15 | 12.5 | 1 | 1.47 | \$36.00 | \$ 52.78 | \$ 13.15 | \$ 19.28 | \$ | 72.06 |
|  |  |  |  |  |  |  |  |  |  |  |  |
| Post-Tensioning Beams |  |  |  |  |  |  |  |  |  |  |  |
|  | - |  |  |  |  |  |  |  |  |  |  |
| $18 \times 22$ |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 19 | 8 | 15.48 | \$36.00 | \$ 557.33 | \$ 13.15 | \$ 203.58 | \$ | 760.91 |
|  |  |  | 30 | 2 | 6.11 | \$36.00 | \$ 220.00 | \$ 13.15 | \$ 80.36 | \$ | 300.36 |
|  |  |  | 40 | 8 | 32.59 | \$36.00 | \$1,173.33 | \$ 13.15 | \$ 428.59 | \$ | 1,601.92 |
|  |  |  | 50 | 9 | 45.83 | \$36.00 | \$1,650.00 | \$ 13.15 | \$ 602.71 | \$ | 2,252.71 |
|  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 50 | 1 | 5.79 | \$36.00 | \$ 208.33 | \$ 13.15 | \$ 76.10 | \$ | 284.43 |
| $22 \times 24$ |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 30 | 1 | 4.07 | \$36.00 | \$ 146.67 | \$ 13.15 | \$ 53.57 | \$ | 200.24 |
|  |  |  | 40 | 1 | 5.43 | \$36.00 | \$ 195.56 | \$ 13.15 | \$ 71.43 | \$ | 266.99 |
| $22 \times 32$ |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 19 | 2 | 6.88 | \$36.00 | \$ 247.70 | \$ 13.15 | \$ 90.48 | \$ | 338.18 |
|  |  |  | 40 | 2 | 14.49 | \$36.00 | \$ 521.48 | \$ 13.15 | \$ 190.49 | \$ | 711.97 |
|  |  |  | 50 | 1 | 9.05 | \$36.00 | \$ 325.93 | \$ 13.15 | \$ 119.05 | \$ | 444.98 |
| $22 \times 36$ |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 19 | 1 | 3.87 | \$36.00 | \$ 139.33 | \$ 13.15 | \$ 50.90 | \$ | 190.23 |
|  |  |  | 40 | 1 | 8.15 | \$36.00 | \$ 293.33 | \$ 13.15 | \$ 107.15 | \$ | 400.48 |
|  |  |  | 50 | 1 | 10.19 | \$36.00 | \$ 366.67 | \$ 13.15 | \$ 133.94 | \$ | 500.60 |
| 24×36 |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 10.15 | 12.5 | 1 | 2.78 | \$36.00 | \$ 100.00 | \$ 13.15 | \$ 36.53 | \$ | 136.53 |
|  |  | 20.25 | 22.5 | 2 | 10.00 | \$36.00 | \$ 360.00 | \$ 13.15 | \$ 131.50 | \$ | 491.50 |
|  |  | 25.30 | 27.5 | 2 | 12.22 | \$36.00 | \$ 440.00 | \$ 13.15 | \$ 160.72 | \$ | 600.72 |
|  |  |  |  |  |  |  |  |  | Total |  | 18,140.50 |


| Forms in Place, Plywood, 2 use |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Size | Length Range (ft.) | Avg. Length (ft.) | Quantity | SFCA | Unit Mat'I Cost | Material Cost | Unit Labor Cost | Labor Cost | Total Cost |  |
| 12 24 24 (t.) |  |  |  |  |  |  |  |  |  |  |
|  | 10.15 | 12.5 | 1 | 31.00 | \$ 1.57 | \$ 48.67 | \$ 5.10 | \$ 158.10 | \$ | 206.77 |
|  |  |  |  |  |  |  |  |  |  |  |
|  | 5.10 | 7.5 | 1 | 21.67 | \$ 1.57 | \$ 34.02 | \$ 5.10 | \$ 110.50 | \$ | 144.52 |
|  | 10.15 | 12.5 | 2 | 31.67 | \$ 1.57 | \$ 49.72 | \$ 5.10 | \$161.50 | \$ | 211.22 |
|  |  |  |  |  |  |  |  |  |  |  |
|  | 0.5 | 7.5 | 1 | 21.67 | \$ 1.57 | \$ 34.02 | \$ 5.10 | \$110.50 | \$ | 144.52 |
|  |  |  |  |  |  |  |  |  |  |  |
|  | 10.15 | 12.5 | 1 | 33.33 | \$ 1.57 | \$ 52.33 | \$ 5.10 | \$ 170.00 | \$ | 222.33 |
|  |  |  |  |  |  |  |  |  |  |  |
|  | 5.10 | 7.5 | 1 | 22.67 | \$ 1.57 | \$ 35.59 | \$ 5.10 | \$ 115.60 | \$ | 151.19 |
| 24×24 |  |  |  |  |  |  |  |  |  |  |
|  | 0.5 | 7.5 | 2 | 23.00 | \$ 1.57 | \$ 36.11 | \$ 5.10 | \$ 117.30 | \$ | 153.41 |
|  | 5.10 | 7.5 | 2 | 23.00 | \$ 1.57 | \$ 36.11 | \$ 5.10 | \$117.30 | \$ | 153.41 |
|  | 10. 15 | 12.5 | 2 | 33.00 | \$ 1.57 | \$ 51.81 | \$ 5.10 | \$168.30 | \$ | 220.11 |
|  | 15.20 | 17.5 | 3 | 43.00 | \$ 1.57 | \$ 67.51 | \$ 5.10 | \$219.30 | \$ | 286.81 |
|  | 20.25 | 22.5 | 10 | 53.00 | \$ 1.57 | \$ 83.21 | \$ 5.10 | \$270.30 | \$ | 353.51 |
|  | 25.30 | 27.5 | 5 | 63.00 | \$ 1.57 | \$ 98.91 | \$ 5.10 | \$ 321.30 | \$ | 420.21 |
| $24 \times 30$ |  |  |  |  |  |  |  |  |  |  |
|  | 10.15 | 12.5 | 1 | 34.00 | \$ 1.57 | \$ 53.38 | \$ 5.10 | \$ 173.40 | \$ | 226.78 |
|  | 15.20 | 17.5 | 1 | 44.00 | \$ 1.57 | \$ 69.08 | \$ 5.10 | \$ 224.40 | \$ | 293.48 |
|  | 20.25 | 22.5 | 4 | 54.00 | \$ 1.57 | \$ 84.78 | \$ 5.10 | \$275.40 | \$ | 360.18 |
|  | 25.30 | 27.5 | 2 | 64.00 | \$ 1.57 | \$ 100.48 | \$ 5.10 | \$ 326.40 | \$ | 426.88 |
| $24 \times 32$ |  |  |  |  |  |  |  |  |  |  |
|  | 10.15 | 12.5 | 1 | 34.33 | \$ 1.57 | \$ 53.90 | \$ 5.10 | \$ 175.10 | \$ | 229.00 |
| $24 \times 57$ |  |  |  |  |  |  |  |  |  |  |
|  | 20.25 | 22.5 | 1 | 58.50 | \$ 1.57 | \$ 91.85 | \$ 5.10 | \$298.35 | \$ | 390.20 |
|  | 25.30 | 27.5 | 1 | 68.50 | \$ 1.57 | \$ 107.55 | \$ 5.10 | \$349.35 | \$ | 456.90 |
| $12 \times 18$ |  |  |  |  |  |  |  |  |  |  |
|  | 5.10 | 7.5 | 4 | 20.00 | \$ 1.57 | \$ 31.40 | \$ 5.10 | \$ 102.00 | \$ | 133.40 |
| $12 \times 24$ |  |  |  |  |  |  |  |  |  |  |
|  | 10.15 | 12.5 | 13 | 31.00 | \$ 1.57 | \$ 48.67 | \$ 5.10 | \$ 158.10 | \$ | 206.77 |
|  | 15.20 | 17.5 | 6 | 41.00 | \$ 1.57 | \$ 64.37 | \$ 5.10 | \$209.10 | \$ | 273.47 |
|  | 20.25 | 22.5 | 6 | 51.00 | \$ 1.57 | \$ 80.07 | \$ 5.10 | \$260.10 | \$ | 340.17 |
| $12 \times 36$ |  |  |  |  |  |  |  |  |  |  |
|  | 10.15 | 12.5 | 4 | 33.00 | \$ 1.57 | \$ 51.81 | \$ 5.10 | \$ 168.30 | \$ | 220.11 |
|  | 15.20 | 17.5 | 2 | 43.00 | \$ 1.57 | \$ 67.51 | \$ 5.10 | \$219.30 | \$ | 286.81 |
|  | 15.20 | 17.5 | 2 | 43.00 | \$ 1.57 | \$ 67.51 | \$ 5.10 | \$219.30 | \$ | 286.81 |
| $12 \times 38$ |  |  |  |  |  |  |  |  |  |  |
|  | 10.15 | 12.5 | 1 | 33.33 | \$ 1.57 | \$ 52.33 | \$ 5.10 | \$170.00 | \$ | 222.33 |
|  |  |  |  |  |  |  |  |  |  |  |
| Post-Tensioning Beams |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
| $18 \times 22$ |  |  |  |  |  |  |  |  |  |  |
|  |  | 19 | 8 | 44.67 | \$ 1.57 | \$ 70.13 | \$ 5.10 | \$227.80 | \$ | 297.93 |
|  |  | 30 | 2 | 66.67 | \$ 1.57 | \$ 104.67 | \$ 5.10 | \$340.00 | \$ | 444.67 |
|  |  | 40 | 8 | 86.67 | \$ 1.57 | \$ 136.07 | \$ 5.10 | \$442.00 | \$ | 578.07 |
|  |  | 50 | 9 | 106.67 | \$ 1.57 | \$ 167.47 | \$ 5.10 | \$544.00 | \$ | 711.47 |
|  |  |  |  |  |  |  |  |  |  |  |
|  |  | 50 | 1 | 6.67 | \$ 1.57 | \$ 10.47 | \$ 5.10 | \$ 34.00 | \$ | 44.47 |
| 22 2 24 |  |  |  |  |  |  |  |  |  |  |
|  |  | 30 | 1 | 67.67 | \$ 1.57 | \$ 106.24 | \$ 5.10 | \$345.10 | \$ | 451.34 |
|  |  | 40 | 1 | 87.67 | \$ 1.57 | \$ 137.64 | \$ 5.10 | \$447.10 | \$ | 584.74 |
| 22×32 |  |  |  |  |  |  |  |  |  |  |
|  |  | 19 | 2 | 47.00 | \$ 1.57 | \$ 73.79 | \$ 5.10 | \$239.70 | \$ | 313.49 |
|  |  | 40 | 2 | 89.00 | \$ 1.57 | \$ 139.73 | \$ 5.10 | \$453.90 | \$ | 593.63 |
|  |  | 50 | 1 | 109.00 | \$ 1.57 | \$ 171.13 | \$ 5.10 | \$555.90 | \$ | 727.03 |
| $22 \times 36$ |  |  |  |  |  |  |  |  |  |  |
|  |  | 19 | 1 | 47.67 | \$ 1.57 | \$ 74.84 | \$ 5.10 | \$243.10 | \$ | 317.94 |
|  |  | 40 | 1 | 89.67 | \$ 1.57 | \$ 140.78 | \$ 5.10 | \$457.30 | \$ | 598.08 |
|  |  | 50 | 1 | 109.67 | \$ 1.57 | \$ 172.18 | \$ 5.10 | \$559.30 | \$ | 731.48 |
| $24 \times 36$ |  |  |  |  |  |  |  |  |  |  |
|  | 10.15 | 12.5 | 1 | 35.00 | \$ 1.57 | \$ 54.95 | \$ 5.10 | \$178.50 | \$ | 233.45 |
|  | 20.25 | 22.5 | 2 | 55.00 | \$ 1.57 | \$ 86.35 | \$ 5.10 | \$280.50 | \$ | 366.85 |
|  | 25.30 | 27.5 | 2 | 65.00 | \$ 1.57 | \$ 102.05 | \$ 5.10 | \$331.50 | \$ | 433.55 |
|  |  |  |  |  |  |  |  | Total |  | ,449.44 |

# Final Report 

## Structural Steel:



| Size |  |  | Length <br> (ft.) | Quantity | L.F. | Unit Mat'l Cost | Material Cost |  | Labor ost | Labor Cost |  | Jnit pment | Equipment Cost | Total Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| HSS $6 \times 6 \times 3 / 8$ |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 23 | 16 | 368 | \$ 880.00 | \$14,080.00 | \$ | 49.00 | \$ 784.00 | \$ | 35.00 | \$ 560.00 | \$ 15,424.00 |
|  |  |  | 15 | 40 | 600 | \$ 880.00 | \$35,200.00 | \$ | 49.00 | \$1,960.00 | \$ | 35.00 | \$1,400.00 | \$ 38,560.00 |
|  |  |  | 13 | 24 | 312 | \$ 880.00 | \$21,120.00 | \$ | 49.00 | \$1,176.00 | \$ | 35.00 | \$ 840.00 | \$ 23,136.00 |
|  |  |  |  |  |  |  |  |  |  |  |  |  | Total | \$77,120.00 |

## Reinforcing:

| Rebar in Concrete |  |  |  |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | Rebar | L.F. | Unit Mat'l <br> Cost | Material Cost | Total |  |  |
| Footing | 6 \#6 | 8670 | $\$$ | 1.35 | $\$ 11,704.50$ | $\$ 11,704.50$ |  |
| Slab on Grade | $\# 4 @ 12 "$ O.C. | 140400 | $\$$ | 0.70 | $\$ 98,280.00$ | $\$ 98,280.00$ |  |
| Elevated Slab | $\# 4 @ 24 "$ O.C. | 69000 | $\$$ | 0.70 | $\$ 48,300.00$ | $\$ 48,300.00$ |  |
| Concrete Column | $8 \# 10$ | 15000 | $\$$ | 3.45 | $\$ 51,750.00$ | $\$ 51,750.00$ |  |
| Concrete Beam | $6 \# 7$ | 17000 | $\$$ | 1.70 | $\$ 28,900.00$ | $\$ 28,900.00$ |  |
|  |  |  |  |  | Total | $\$ 238,934.50$ |  |

## Appendix F: General Conditions

| General Conditions Estimate |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Description | Unit | Quantity | Cost/Unit |  | Total |
| Field Personnel |  |  |  |  |  |
| Project Manager | Week | 250 | $\begin{gathered} \$ \\ 1,925.00 \end{gathered}$ | \$ | 481,250.00 |
| Superintendent | Week | 250 | $\begin{gathered} \$ \\ 1,775.00 \end{gathered}$ | \$ | 443,750.00 |
| Asst. Superintendent | Week | 250 | $\begin{gathered} \$ \\ 1,600.00 \end{gathered}$ | \$ | 400,000.00 |
| Asst. Superintendent | Week | 250 | $\begin{gathered} \$ \\ 1,600.00 \end{gathered}$ | \$ | 400,000.00 |
| Field Engineer | Week | 250 | $\begin{gathered} \$ \\ 1,165.00 \\ \hline \end{gathered}$ | \$ | 291,250.00 |
| Asst. Field Engineer | Week | 250 | $\begin{gathered} \$ \\ 895.00 \end{gathered}$ | \$ | 223,750.00 |
| Asst. Field Engineer | Week | 250 | $\begin{gathered} \$ \\ 895.00 \\ \hline \end{gathered}$ | \$ | 223,750.00 |
| General Expenses |  |  |  |  |  |
| Field Trailer 32'x8' | Mo | 60 | $\begin{gathered} \$ \\ 200.00 \end{gathered}$ | \$ | 12,000.00 |
| Office Equipment | Mo | 60 | $\begin{gathered} \$ \\ 155.00 \end{gathered}$ | \$ | 9,300.00 |
| Office Supplies | Mo | 60 | $\begin{gathered} \$ \\ 85.00 \end{gathered}$ | \$ | 5,100.00 |
| Office Telephone | Mo | 60 | $\begin{gathered} \$ \\ 80.00 \end{gathered}$ | \$ | 4,800.00 |
| Office Lights and HVAC | Mo | 60 | $\begin{gathered} \$ \\ 150.00 \end{gathered}$ | \$ | 9,000.00 |
| Temporary Fencing, 6' high | L.F. | 30 | $\begin{gathered} \$ \\ 9.44 \end{gathered}$ |  | $\begin{gathered} \$ \\ 283.20 \end{gathered}$ |
| Toilet 1, portable | Mo | 60 | $\begin{gathered} \$ \\ 150.00 \end{gathered}$ | \$ | 36,000.00 |
| Toilet 2, portable | Mo | 60 | $\begin{gathered} \$ \\ 150.00 \\ \hline \end{gathered}$ | \$ | 36,000.00 |
| Toilet 3, portable | Mo | 60 | $\begin{gathered} \$ \\ 150.00 \end{gathered}$ | \$ | 36,000.00 |
| Permits | Job | 1 | 0.50\% | \$ | 465,000.00 |
| Final Clean Up | Job | 1 | 0.30\% | \$ | 279,000.00 |

# Final Report 

| Temporary Utilities |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Temporary Lighting, 4 floors | CSF/FIr. | 581 | $\begin{gathered} \$ \\ 13.68 \end{gathered}$ | \$ | 31,792.32 |
| Temporary Heating, 4 floors | CSF/FIr | 581 | $\begin{gathered} \$ \\ 30.27 \end{gathered}$ | \$ | 70,347.48 |
| Temporary Power, 4 floors | CSF/FIr. | 581 | $\begin{gathered} \$ \\ 47.75 \end{gathered}$ | \$ | 110,971.00 |
| Insurance |  |  |  |  |  |
| Insurance, All-risk type | Job | 1 | 0.25\% | \$ | 232,500.00 |
| Performance Bond | Job | 1 | 0.60\% | \$ | 558,000.00 |
| Scheduling, Large job | Job | 1 | 0.03\% | \$ | 27,900.00 |
| Permits, Rule of thumb | Job | 1 | 0.50\% | \$ | 465,000.00 |
|  |  |  | Sub-Total | \$ | 4,852,744.00 |
|  |  | Location Factor |  |  | 0.982 |
|  |  | Total |  | \$ | 4,765,394.61 |

## Appendix G: Detailed Project Schedule



| Final Report | 2010 |
| :--- | :--- |



Final Report $\mid 2010$


## Appendix H: Proposed Schedule



Final Report 2010


Final Report 2010


## Appendix I: Proposed General Conditions



## Appendix J: Proposed Security Cost

| General Conditions Estimate |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Description | Unit | Quantity | Cost/Unit |  | Total |
| Temporary Security |  |  |  |  |  |
| Watchman | Hr | 2580 | $\begin{gathered} \$ \\ 25.00 \end{gathered}$ | \$ | 64,500.00 |
| Watchman, Overtime | Hr | 4644 | $\begin{gathered} \$ \\ 37.50 \end{gathered}$ | \$ | 174,150.00 |
|  |  |  | Sub-Total | \$ | 238,650.00 |
|  |  | Location Factor |  |  | 0.982 |
|  |  | Total Cost |  | \$ | 234,354.30 |

Final Report

## Appendix K: Shrub Replacement Plan




## Appendix L: Irrigation System Specifications

## Below Ground Cistern Tank Collection System


(Note how overflow is directed away from and downhill from the tank excavation and housing structure and must drain at the surface).
(Follow underground cistern installation instructions carefully which are included with each tank.)

The Complete System package detailing a 1200 gallon underground cistern (PN10010-).
A. Leaf Eater (RCLE3).
B. First Flush In-Ground Diverter (RCID12).
C. $4^{*}$ Overflow (RCOF4).
D. $12^{\prime \prime}$ Riser.
E. Submersible Pump (RCSUBPUMP) with controls (RCCONTROL).
F. Filter Pit (optional) (RCFP).
G. 3 Cord Seal (RC3CS).

Snyder Industries has the solution for your rain harvesting needs. Snyder is a leading manufacturer of exceptionally performing polyethylene water tanks and offers a full line of rain harvesting systems and components.

## Potential Annual Rainwater Collection

| Rainfall <br> in <br> Inches | Square Feet of Roof Surface |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1,000 | 2,000 | 3,000 | 4,000 | 5,000 | 10,000 |  |
|  | Gallons of Water |  |  |  |  |  |  |
| 1 | 625 | 1,250 | 1,875 | 2,500 | 3,125 | 6,250 |  |
| 2 | 1,250 | 2,500 | 3,750 | 5,000 | 6,250 | 12,500 |  |
| 5 | 3,125 | 6,250 | 9,375 | 12,500 | 15,625 | 31,500 |  |
| 10 | 6,250 | 12,500 | 18,750 | 25,000 | 31,250 | 62,500 |  |
| 30 | 18,750 | 37,500 | 56,250 | 75,000 | 93,750 | 187,500 |  |
| 40 | 25,000 | 50,000 | 75,000 | 100,000 | 125,000 | 250,000 |  |
| 50 | 31,250 | 62,500 | 93,750 | 125,000 | 156,250 | 312,500 |  |
| 60 | 37,500 | 75,000 | 112,500 | 150,000 | 187,500 | 375,000 |  |
|  |  | Gallons of Water Captured |  |  |  |  |  |

Calculation: annual rainfall per inch x 625 gallons per 1,000 square feet roof surface.
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## Appendix M: Rainwater Collection Areas



